RANCHO MURIETA COMMUNITY SERVICES DISTRICT



15160 JACKSON ROAD RANCHO MURIETA, CALIFORNIA 95683

SPECIAL BOARD MEETING January 30, 2017 at 1:00 p.m.

NOTICE IS HEREBY GIVEN that the President of the Board of Directors of the Rancho Murieta Community Services District has called a Special Meeting of the Board to be held on January 30, 2017 at 1:00 p.m. at the Rancho Murieta Community Services District Board Room at 15160 Jackson Road, Rancho Murieta.

AGENDA

- 1. CALL TO ORDER, ROLL CALL Determination of Quorum President Pecotich (Roll Call)
- 1:00

- 2. ADOPT AGENDA (Motion)
- 3. COMMENTS FROM THE PUBLIC

For this Special Meeting, members of the public may **ONLY** comment on items specifically agendized. Members of the public wishing to address a specific agendized item are encouraged to offer their public comment during consideration of that item. With certain exceptions, the Board may not discuss or take action on items that are not on the agenda.

If you wish to address the Board at the time of the agendized item, as a courtesy, please state your name and address, and reserve your comments to no more than 3 minutes so that others may be allowed to speak. (5 min.)

- 4. REVIEW RECYCLED WATER PLAN AND PRE-DESIGN REPORT (Discussion/Action) (Motion)
- 5. DIRECTOR COMMENTS/SUGGESTIONS
- 6. ADJOURNMENT (Motion)

"In accordance with California Government Code Section 54957.5, any writing or document that is a public record, relates to an open session agenda item and is distributed less than 72 hours prior to a regular meeting, will be made available for public inspection in the District offices during normal business hours. If, however, the document is not distributed until the regular meeting to which it relates, then the document or writing will be made available to the public at the location of the meeting."

In compliance with the Americans with Disabilities Act, if you are a disabled person and you need a disability-related modification or accommodation to participate in this meeting, please contact the District Office at 916-354-3700 or fax 916-354-2082. Requests must be made as soon as possible and at least two (2) full business days before the start of the meeting.

Note: This agenda is posted pursuant to the provisions of the Government Code commencing at Section 54950. The date of this posting is January 27, 2017. Posting locations are: 1) District Office; 2) Rancho Murieta Post Office; 3) Rancho Murieta Association; 4) Murieta Village Association.

MEMORANDUM

Date: January 27, 2017

To: Board of Directors

From: Darlene J. Thiel, General Manager

Subject: Review Recycled Water Plan and Pre-Design Report

RECOMMENDED ACTION

No action.

BACKGROUND

The purpose of this Recycled Water Plan Workshop is to briefly review the history behind the District's activity in establishing the use of recycled water and the policy decisions to implement the use of recycled water for outside irrigation for new development. We will also review and discuss the preliminary Pre-Design Report developed by Kevin Kennedy, Kennedy/Jenks. Kevin will make a presentation on the Pre-Design Report at the workshop and will be available for questions and discussion regarding the report.

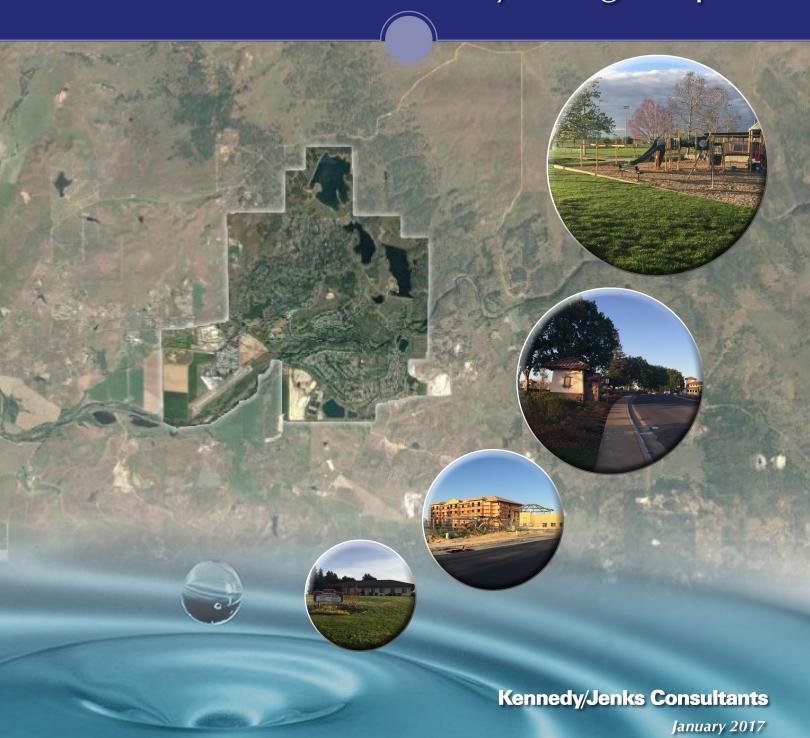
The goal is to bring a final version of the Pre-Design Report to the Board of Directors for adoption at either the February or March 2017 Regular Board Meeting.



Rancho Murieta Community Services District

DRAFT

Recycled Water Program Preliminary Design Report



Recycled Water Program Preliminary Design Report

January 2017

Draft



Prepared under the responsible charge of

Kevin Kennedy Registration No. C61206

Kennedy/Jenks Consultants

10850 Gold Center Drive, Suite 350 Rancho Cordova, California 95670

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Recycled Water Program Preliminary Design Report (Draft)

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List of Abbreviations and Acronyms

ac Acres

ACP asbestos cement pipe

ADWF average dry weather flow

AFY acre-feet per year

CCB chlorine contact basin

CCP chlorine contact pipe

DAF dissolved air flotation

DIP ductile iron pipe

District Rancho Murieta Community Services District

ENR Engineering News Record

ft Feet

gpm gallons per minute

HGL Hydraulic Grade Line

In Inch

IPR Indirect Potable Recharge

IS/MND Initial Study/Mitigated Negative Declaration Rancho Murieta Recycled

Water System Expansion Project (AECOM, June 2014)

LF linear feet

MCCs motor control centers

MGD million gallons per day

mm Millimeter

NOA Naturally Occurring Asbestos

NPDES National Pollution Discharge Elimination System

PDR Preliminary Design Report (this document)

PLC Programmable Logic Controller

Recycled Water Program Preliminary Design Report (Draft)

PRV pressure reducing valve

psi pounds per square inch

RMA Rancho Murieta Association

RMCC Rancho Murieta Country Club

RMPI Rancho Murieta Properties, Inc.

RVs recreational vehicles

SCADA Supervisory Control and Data Acquisition

TDH Total dynamic head

WDR Waste Discharge Requirements

WWRP Wastewater Reclamation Plant

WWTF Wastewater Treatment Facility

Executive Summary

The purpose of this Preliminary Design Report (PDR) is to describe Phase 1 and Buildout of Rancho Murieta Community Services District's (District's) Recycled Water Program with respect to existing and future conditions; development projections, phasing and recycled water use areas; recommended improvements and descriptions (including costs and timeline) and implementation plan. This PDR will also serve as the basis for subsequent environmental, regulatory permitting activities and detailed design and construction efforts associated with the recommended Phase 1 Recycled Water Improvements Project. In addition, this PDR also describes the approximate timeline for the improvements required for Buildout of the District's Recycled Water Program. Refinements and adjustment to the recommended Buildout improvements are expected to be conducted later as the implementation timeline draws closer and/or if development plans change.

Existing recycled water use areas can accommodate the equivalent of roughly 3,265 residential homes based on the 0.5 MGD ADWF capacity described in the WDR.1 Review and comparison of the 3,265 equivalent residential homes to the development projections indicate the need to expand recycled water use beyond the North and South Golf Courses in the near future to accommodate growth. The projected average dry weather flow (ADWF) at Buildout is 0.8 MGD. The ADWF is currently about 0.34 MGD.

Development projections obtained from the District's Water Supply Assessment Technical Memorandum (Maddaus Water Management, Inc., January 18, 2016) and updated information obtained from developers indicate that the District's current rated ADWF of 0.5 MGD is projected to be exceeded in 2019. However this development timeline is consider both aggressive and optimistic compared to historic growth patterns. Actual development rates will likely be lower and the development timeline extended beyond the year 2035.

A series of improvements is recommended to provide the capacity needed to accommodate growth. Table ES-1 presents a summary of the recommended improvements and estimated costs.

Figure ES-1 presents a summary of recommended implementation activities, timelines and deadlines for Phase 1 improvements. Buildout improvements are anticipated to require about 3 years to complete. Flows are projected to approach the rated ADWF capacity of the existing seasonal storage reservoirs around 2023. Therefore, the District should initiate the expansion of the seasonal storage reservoirs no later than January 2020.

¹ 0.5 MGD flow includes allocations for infill (0.05 MGD), Murieta Gardens (residential and commercial) and The Retreats (residential) for a total of 3,265 total equivalent residential units.

Table ES-1. Recommended Recycled Water Improvements and Estimated Costs

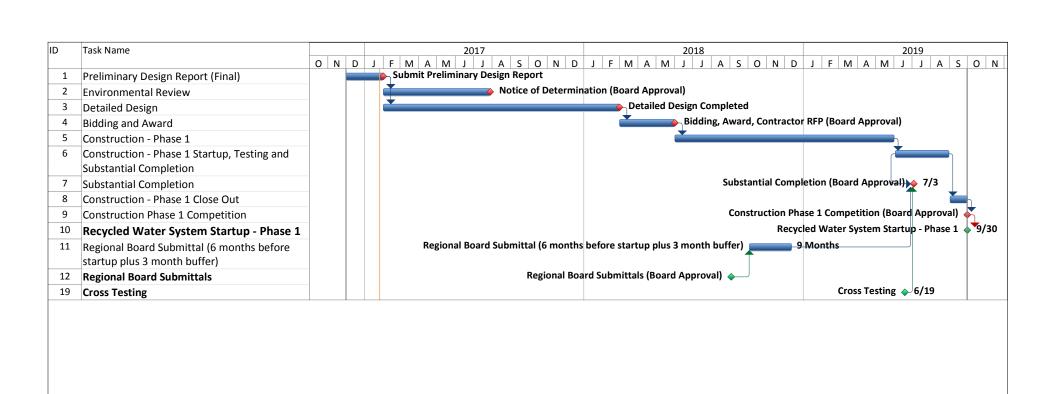
No.	Improvement	Estimated Cost (\$)a				
NU.	Phase 1 Recycled Water Improvement	`,				
1	Recycled Water SCADA Control System	250,000				
2	Equalization Basin Potable Water Air Gap	76,000				
3	Recycled Water Pumping Station	1,045,000				
4	District Headquarters Conversion	20,000				
5	Northwest Recycled Water Transmission Main	1,441,000				
6	Lookout Hill Booster Pumping Station	612,000				
7	Escuela Park Conversion	16,000				
8	Stonehouse Park Conversion	36,000				
9	Lookout Hill Recycled Water Storage Tank	545,000				
10	North Main Gate Conversion	18,000				
11	Commercial Loop Conversion	TBD				
11	Phase 1 Subtotal (Estimated Construction Cost)	4,060,000				
12	Soft Costs – 32.5% (Admin., Reg., Eng., Construct Man.)	1,319,500				
12	Phase 1 Total (Project Cost)	5,380,000 ^b				
	Buildout Recycled Water Improvements					
13	SCADA Upgrades	82,000				
14	Disinfection Facilities Upgrade	665,000				
15	North Golf Course Conveyance System	1,620,000				
16	Bass Lake Tank	1,216,000				
17	Bass Lake Booster Pumping Station	625,000				
18	Seasonal Storage Reservoir Expansion	830,000				
19	Van Vleck Sprayfield 4	890,000				
20	DAF Pumping Replacement	100,000				
20	Buildout Subtotal (Estimated Construction Cost)	6,030,000°				
21	Soft Costs – 33% (Admin., Reg., Eng., Construct Man.)	1,960,000				
	Buildout Total (Project Cost)	7,990,000				
	Phase 1 and Buildout Recycled Water Impi					
	Grand Total (Phase 1 and Buildout)	13,400,000d				
	Estimated Number of New Equivalent Residential Units	2,213				
	Estimated Cost per Connection (\$/ERU)	\$6,055				
	Localitation Cost per Connection (4/ Litto)	ΨΟ,ΟΟΟ				

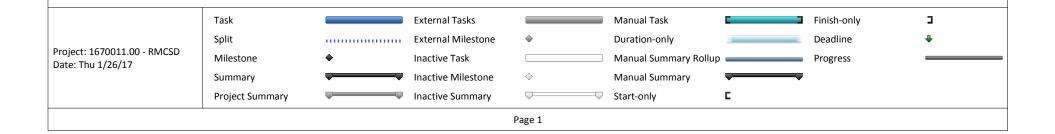
^a Estimated costs based upon Engineering News Record (ENR) 20 City Average Construction Cost Index (CCI) at 10,385 (August 2016)

^b Compared to \$10,014,000 (\$9,100,000 adjusted for inflation) as described previously in the District's Title XVI Recycled Water Feasibility Study

^c Compared to \$15,055,000 as described previously in the District's Title XVI Recycled Water Feasibility Study

d Compared to \$25,070,000 as described previously in the District's Title XVI Recycled Water Feasibility Study





Recycled Water Program Preliminary Design Report (Draft)

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Section 1: Introduction

This Preliminary Design Report (PDR) describes Phase 1 and Buildout of Rancho Murieta Community Services District's (District's) Recycled Water Program with respect to existing and future conditions; development projections, phasing and recycled water use areas; recommended improvements and descriptions (including costs and timeline) and implementation plan.

This section presents and describes the Recycled Water Program background, objectives, benefits, PDR organization, development projections, and acknowledgements.

1.1: Background and Objectives

The District's existing recycled water use areas (i.e., the North and South Golf Courses) can accommodate roughly 3,265 equivalent residential units² based on the 0.5 million gallons per day (MGD) average dry weather flow (ADWF) capacity described in the District's Waste Discharge Requirements (WDR). Review and comparison of this 3,265 equivalent residential units limitation to the current development projections indicate the need to expand recycled water use to accommodate projected development within Rancho Murieta. Recycled water use provides disposal and beneficial reuse of the treated wastewater effluent required to accommodate future planned development.

The District's Board of Directors adopted the Recycled Water Standards (October, 2013) in response to the adoption of District Policy No. 2011-07, Authorized and Mandated Use of Recycled Water (Recycled Water Policy) and the adoption of District Code, Chapter 17, Recycled Water Code (Recycled Water Code). The Recycled Water Policy requires the use of recycled water wherever economically and physically feasible as determined by the Board and identified, in general, that the lands subject to mandatory recycled water use are the undeveloped parcels within the existing District service area. Specific future developments areas were further designated³ within the existing District service area and the District's off-site disposal area on the neighboring Van Vleck Ranch. Expanded recycled water use at specific future sites is estimated and anticipated to provide the District with the increased disposal and beneficial reuse of treated wastewater effluent required to serve future developments, accommodate growth within Rancho Murieta, provide an offset to potable water demands, and comply with the WDR.

The objectives of this PDR are to describe Phase 1 and Buildout needs/requirements of the District's Recycled Water Program with respect to existing and future conditions; development projections, phasing and recycled water use areas; recommended improvements and descriptions (including costs and timeline) and implementation plan. Table 1 presents a listing of the proposed Phase 1 and Buildout future developments and recycled water use areas. Figure 1 shows existing developments along with proposed developments for Phase 1 and Buildout.

This PDR will serve as the basis for subsequent environmental, regulatory permitting activities, and detailed design and construction efforts associated with the recommended Phase 1 Recycled Water Improvements Project described in Section 3 of this PDR. In addition, this PDR also describes the approximate timeline for the improvements required for Buildout of the District's Recycled Water Program. Refinements and adjustment to the recommended Buildout improvements are expected

1

² 0.5 MGD flow includes allocations for infill (0.05 MGD), Murieta Gardens (residential and commercial) and The Retreats (residential) for a total of 3,265 total equivalent residential units.

³ Within the District's submittal of the Report of Waste Discharge and subsequent adoption of the Master Reclamation Permit (December 20, 2013).

to be conducted later as the implementation timeline draws closer and/or if development plans change.

Table 1. Proposed Developments and Recycled Water Use Areas

Phase	Proposed Developments	Proposed Recycled Water Use Areas
Phase 1	Murieta Gardens	Murieta Gardens ^a [U, R]
	Retreats (North, West and East)	Retreats ^a (North, West and East) [U]
		Stonehouse Parkb (existing) [U]
		Escuela Park ^b (existing) [U]
		Main Northgate ^b (existing) [U]
		District Office ^b (existing) [U]
		Commercial Loop ^c
Buildout	Residences of Murieta Hills	Residences of Murieta Hills ^a [U,R]
	Apartments	Apartments ^a [U]
	Industrial/Commercial/Residential	Industrial/Commercial/Residential ^a [U,R]
	Village A	Village A ^a [R]
	Village B	Village B ^a [R]
	Village C	Village C ^a [R]
	Village D	
	Village E	
	Village F	
	Village G	
	Village H	
	Riverview	
	Lakeview	

^a As requested by the District Board at the December 16, 2015 Board meeting.

Phase 1 and Buildout of the District's Recycled Water Program consists of a series of improvements to the District's existing Wastewater Reclamation Plant (WWRP) and North Golf Course recycled water conveyance system⁴ to serve future residential developments, existing parks, common areas and other landscaping consistent with the District's adopted Recycled Water Code, Recycled Water Standards and Waste Discharge Requirements. Ultimately, the District's expanded Recycled Water Program will provide the disposal capacity needed to accommodate future developments and offset (reduce) potable water demands by approximately 335 acre-feet per year (AFY).

For the purposes of this report, future reuse areas have been categorized in Table 1 according to the following definitions:

• Residential Recycled Water Irrigation [R]: Future recycled water front and backyard irrigation of future residential development landscaping consistent with the District's adopted Recycled Water Code, Recycled Water Standards and Waste Discharge Requirements. As indicated in Table 1, there are six developments that have use areas which fall within this category.

2

^b As requested by District staff for October 10, 2016 Improvements Committee presentation *and if deemed to be cost effective by the District Board.*

^c Recycled water service to these existing urban irrigation areas appears to be cost effective. However, discussions with the owner are recommended prior to moving forward.

U = urban recycled water irrigation, see definition below

R = residential recycled water irrigation, see definition below

⁴ Originally owned by RMPI, now Rancho Murieta Properties, LLC., and operated by Rancho Murieta Country Club (RMCC) as described in Section 2.

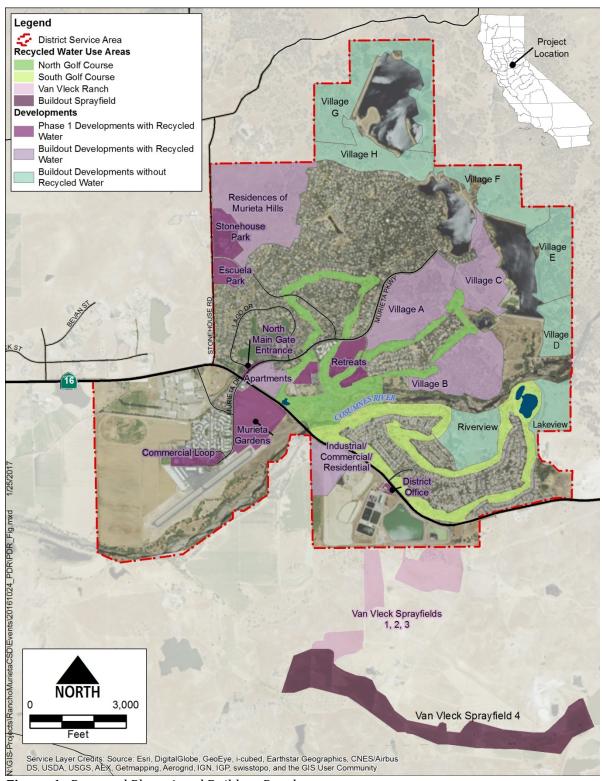


Figure 1. Proposed Phase 1 and Buildout Developments

• Urban Recycled Water Irrigation [U]: Future recycled water irrigation of existing parks, common areas and other landscaping consistent with the District's adopted Recycled Water Code, Recycled Water Standards and Waste Discharge Requirements. As indicated in Table 1, there are nine developments that have use areas which fall within this category.

Phase 1 of the District's Recycled Water Program could be initiated as early as mid-2019 as described later in the last section of the PDR. According to development projections provided by developers/owners of the remaining undeveloped parcels within the District's service area, Buildout is projected to occur in the 2035 timeline as described later in Section 2.3.

The following documents, reports, studies, etc., (presented in sequence) were used in the development of this PDR:

- Agreement for Availability and Use of Reclaimed Wastewater, May 17, 1988
- Amendment to Agreement for Availability and Use of Reclaimed Wastewater, May 4, 1994
- Rancho Murieta North Infrastructure Master Plan (MacKay & Somps, May 2003)
- Recycled Water Code, District Code Chapter 17 (Rancho Murieta Community Services District, January 8, 2012)
- Title XVI Recycled Water Feasibility Study (AECOM, June 2014)
- Initial Study/Mitigated Negative Declaration Rancho Murieta Recycled Water System Expansion Project (AECOM, June 2014)
- California Regional Water Quality Control Board, Central Valley Region Order No. R5-2014-149 Wastewater District Requirements and Master Recycling Permit (WDR)
- USBR Funding Application (AECOM, January 13, 2016)
- Water Supply Assessment Technical Memorandum (Maddaus Water Management, Inc., January 18, 2016)
- Retreats West Capacity Certification Letter (Kennedy/Jenks Consultants, May 4, 2016)
- Draft Sewer Study for the Retreats North & East (Baker-Williams Engineering Group, May 6, 2016)
- Draft Sewer Study for Murieta Gardens I & II (Baker-Williams Engineering Group, May 15, 2016)
- Preliminary Sewer Study for Rancho Murieta North (Baker-Williams Engineering Group, May 31, 2016)
- Draft Recycled Water Modeling Study (AECOM, June 2016)

1.2: Development Projections

Buildout is projected to occur around 2035 based on the latest development projections and result in roughly 4,817 equivalent residential units⁵ within the District's service area. Figure 2 graphically illustrates a summary of development and associated ADWF projections. The level of development reflects an increase of roughly 85 percent above the current number of equivalent residential units.

Review and analyses of the development projections indicate the following distinct periods of different projected rates of growth:

- 2016 through 2020: Approximately 1,355 new equivalent residential units (11%/yr. growth rate)
- 2020 through 2030: Approximately 490 new residential homes (1.2%/yr. growth rate)

⁵ Value includes future commercial and/or industrial connections and is based on 2,604 existing equivalent residential units.

- 2030 through 2035: Approximately 370 new residential homes (1.7%/yr. growth rate)
- 2035 through 2045: At Buildout, no new homes (0%/yr. growth rate thereafter)

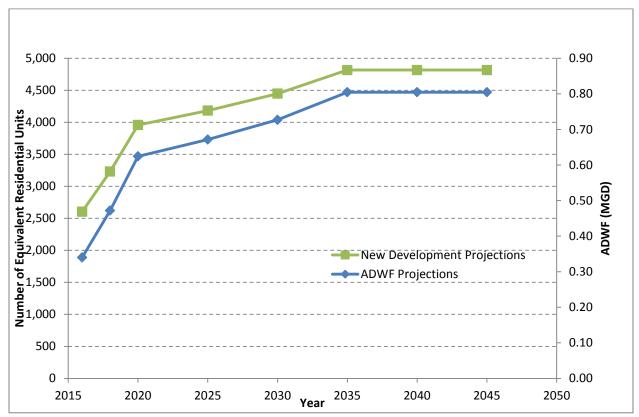


Figure 2. Phase 1 and Buildout Development and ADWF Projections

Existing recycled water use areas (i.e., North and South Golf Courses) can accommodate roughly 3,265 equivalent residential units based on the 0.5 MGD ADWF capacity described in the WDR⁶. Review and comparison of the 3,265 equivalent residential units to the development projections shown in Figure 2 indicate the need to expand recycled water use beyond the North and South Golf Courses in the future to support the level of development currently projected for Rancho Murieta.

1.3: Program Benefits

The District's Recycled Water Program is aligned with the actions needed to (1) provide additional water to help offset California's dwindling water supplies, (2) aggressively promote and demonstrate water programs that stretch California's available potable water supplies, and (3) contribute to the long-term recovery of the Canal Basin and Delta and Cosumnes River ecosystems. The District's Recycled Water Program will:

- Leverage and apply the District's Recycled Water Program Codes, Standards, requirements, etc.
- Offset potable water demands, conserve surface water supplies and reduce Cosumnes River diversions (335 AFY).
- Provide a beneficial, sustainable and long-term means for treated effluent disposal.
- Help the District meet their 20x2020 Water Conservation Goals.
- Increase water supply reliability and reduce drought deficits.

 $^{^6}$ 0.5 MGD flow includes allocations for infill (0.05 MGD), Murieta Gardens (residential and commercial) and The Retreats (residential) for a total of 3,265 total equivalent residential units.

- Maximize use of existing infrastructure.
- Provide opportunities to serve other potential users along the recycled water transmission pipeline alignments.
- Reduce the potential need to upgrade the District's existing Wastewater Treatment Facility (WWTF) and WWRP to more modern and conventional facilities that may have been otherwise required for surface water discharge via request and approval of a National Discharge Elimination System (NPDES) permit or Indirect Potable Reuse (IPR).

1.4: Preliminary Design Report Organization

This PDR has been organized as follows:

- Executive Summary
- Section 1. Introduction (this section)
- Section 2. Basis of Design
- Section 3. Recommended Improvements
- Section 4. Project Implementation

1.5: Acknowledgements

We appreciate and would like to thank the Rancho Murieta Community Services District for providing the opportunity to develop this PDR and work on their Recycled Water Program. We appreciate and acknowledge the efforts of the District staff, most notably Darlene Thiel, General Manager and Paul Siebensohn, Director of Field Operations, along with the Board of Directors. Without their input and support, this PDR could not have been developed to its current point of completion.

Section 2: Basis of Design

This section presents the basis of design, assumptions and a summary of the system requirements recommended for Phase 1 and Buildout of the District's Recycled Water Program. Development projections, wastewater production and recycled water demand estimates, hydraulic modeling and other calculations used to establish design criteria can be found in the Appendix.

2.1: Service Area

Rancho Murieta is located approximately 20 miles east of Sacramento on State Highway 16. The area served by the District is illustrated in Figure 3 and encompasses approximately 3,500 acres. Land uses within the District service area include approximately 2,000 acres for single family residences, townhouses, apartments, duplexes and mobile homes. In January 2016, when the District's Water Supply Assessment was adopted by the Board, the District served 2,604 metered connections comprised of 2,502 residential, 97 commercial and 5 park connections. Local parks are currently being irrigated with potable water. According to Sacramento County's approved Planned Unit Development Plan at Buildout, the development of the District's service area potentially represents roughly 5,189 residential units. However as described in the previous section, recent development plans reflect a lower number of connections at Buildout than Sacramento County's approved Planned Unit Development Plan.

2.2: District Recycled Water Code and Standards

With respect to wastewater collections treatment and disposal, the District falls within the jurisdiction of the Central Valley Regional Water Quality Control Board (Regional Board), whose mission is to preserve, enhance, and restore the quality of California's water resources and to ensure their proper allocation and efficient use for the benefit of present and future generations. A specific goal of the Regional Board is to promote and expand the beneficial use of recycled water. In an effort to support this goal, where applicable, the District has chosen to serve recycled water to future customers, where deemed to be cost effective and to protect, preserve, and conserve ground and surface water resources within the District's service area.

The District's Board of Directors adopted the Recycled Water Standards (October, 2013) in response to the adoption of District Policy No. 2011-07 Authorized and Mandated Use of Recycled Water (Recycled Water Policy) and the adoption of District Code, Chapter 17, Recycled Water Code (Recycled Water Code). The Recycled Water Policy requires the use of recycled water wherever economically and physically feasible as determined by the Board and identified, in general, that the lands subject to mandatory recycled water use are the undeveloped parcels within the existing District service area. Specific future developments areas were further designated⁸ within the existing District service area and the District's off-site disposal area on the neighboring Van Vleck Ranch. Expanded recycled water use at specific future sites is estimated and anticipated to provide the District with the increased disposal and beneficial reuse of treated wastewater effluent required to serve future developments, accommodate growth within Rancho Murieta, provide an offset to potable water demands, and comply with the WDR.

The District's Recycled Water Standards were developed to establish procedures and minimum standards, specifications and limitations to ensure the health, safety, and general welfare of the citizens of Rancho Murieta when installing infrastructure for, and the use of, recycled water, consistent with the laws and regulations of the State of California, as well as to ensure uniformity in

⁷ Since January 2016, there has been an increase of 32 residential units; equivalent to roughly a 0.12% per year growth rate.

 $^{^8}$ With the District's submittal of the Report of Waste Discharge and subsequent adoption of the Master Reclamation Permit (December 20, 2013).

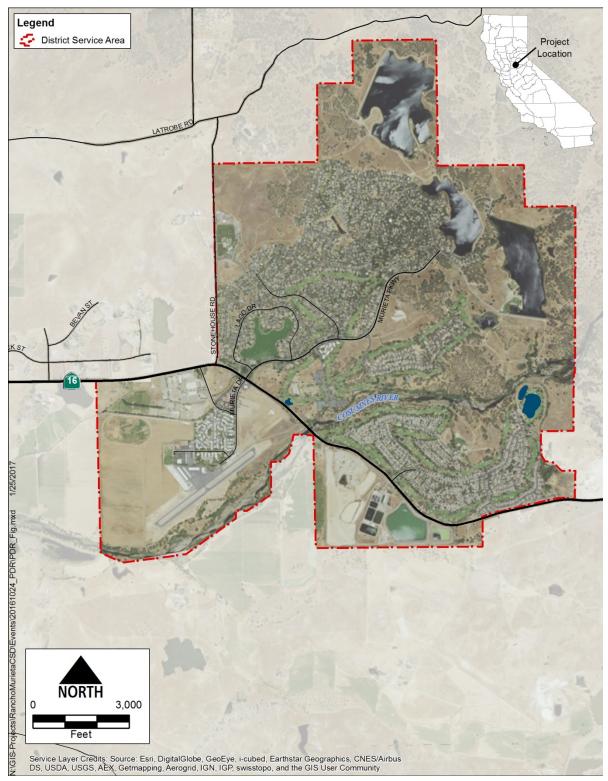


Figure 3. District Service Area Boundary

infrastructure design, format, methodology, construction materials, and quality of work products of the facilities associated with the expanded recycled water system. The Recycled Water Standards are intended to assist recycled water use applicants, authorized contractors, customers, and design consultants with the planning, design, repair, and construction of the expanded recycled water system and were intended to be consistent and ensure compliance with the District's Recycled Water Code and other governing policies, instructions, and regulations related to the use of recycled water. Aspects of the District's Recycled Water Standards applicable to the expanded recycled water system include the general guidelines (e.g., general requirements, system responsibilities, user liability and responsibility, recycled water infrastructure and service, etc.), design and construction standards⁹ and standard details.

2.3: Existing and Proposed Developments

Existing and future proposed Phase 1 and Buildout developments are shown in Figure 4 and Figure 5, respectively, and their assumed timelines are provided below in Table 2. The following sections describe proposed future developments. Estimated wastewater production and recycled water demand estimates were either obtained from the latest development-specific sewer studies or the Title XVI Recycled Water Feasibility Study.

Table 2. Summary of Future Development Timelines¹⁰

Development and Phase ¹		Percent of Future Homes Occupied (%) ¹						
Development and Phase	2018	2020	2025	2030	2035	2040	2045	
Murieta Gardens (305)	Phase 1	100						
The Retreats (88)	Phase 1	100						•
Village A (167)	Buildout		70	15	7	8		
Village B (167)	Buildout		10	30	30	30		••••
Village C (130)	Buildout		10	40	40	10		•
Village D (42)	Buildout			25	25	50		
Village E (43)	Buildout				20	80		
Village F (95)	Buildout			2	38	60		•
Village G (53)	Buildout				10	90		
Village H (122)	Buildout			10	25	65		
Apartments (170)	Buildout		70	15	7	8		
Residences of Murieta Hills (198)	Buildout		100					•
Lakeview (99)	Buildout		100					
Riverview (140)	Buildout		100					
Industrial/Commercial/ Residential (160)	Buildout		15	30	30	25		

Developments to be served recycled water

Developments not to be served recycled water

¹Values shown are percentages and represent the percent of total number of equivalent residential units estimated to be constructed and/or occupied by the referred date. Values shown in parentheses () represent the number of equivalent residential units to be added.

⁹ Where applicable given the expectation of reusing or re-purposing existing pipelines.

¹⁰ Village A through H, Apartments and Industrial/Commercial/Residential timelines obtained from the District's Water Supply Assessment. Lakeview, Riverview, and Residences of Murieta Hills development timelines based on discussions with Les Hock of Hock Construction Management Inc. Timelines for Murieta Gardens and The Retreats obtained from Murieta Gardens I & II Sewer Study and The Retreats North & East and The Retreats West Sewer Studies.

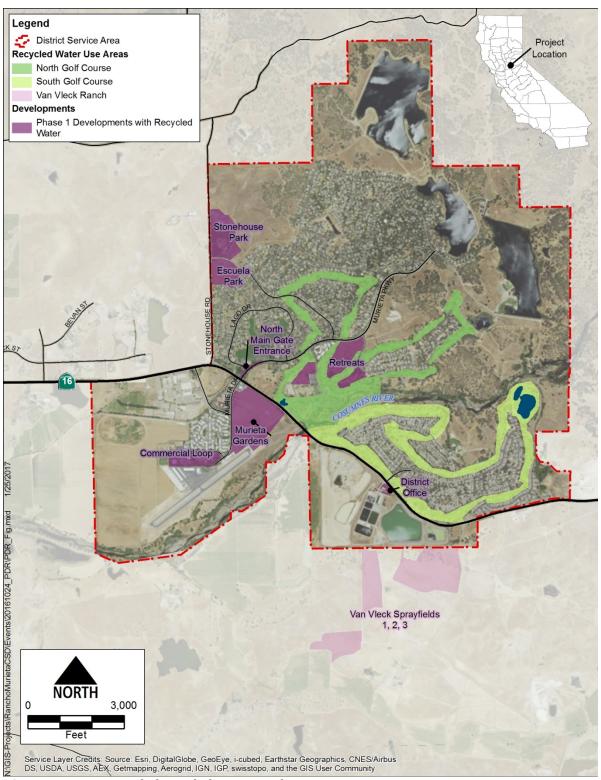


Figure 4. Existing and Planned Phase 1 Developments

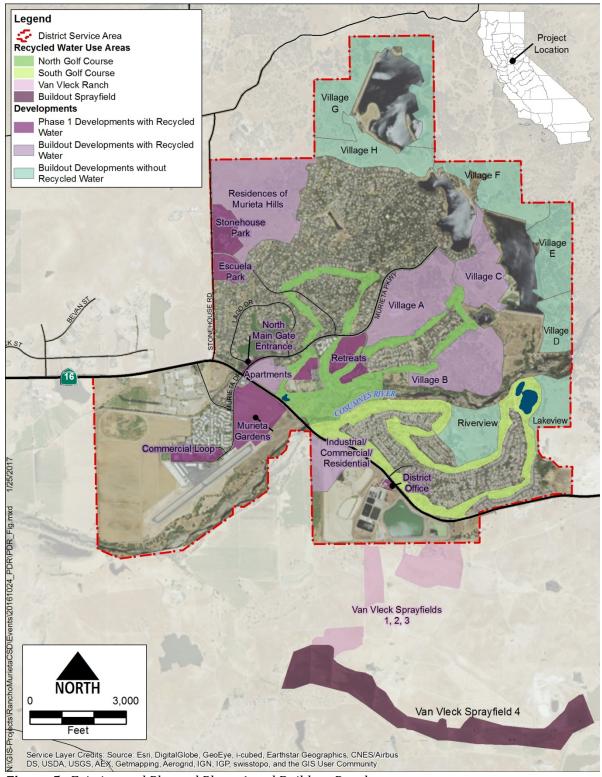


Figure 5. Existing and Planned Phase 1 and Buildout Developments

2.3.1: Murieta Gardens (Phase 1)

As described in the May 15, 2016 Sewer Study, Murieta Gardens is a Phase 1 development consisting of mixed use commercial development (Murieta Gardens I) and a residential development (Murieta Gardens II) located southeast of the intersection of Highway 16 (Jackson

Highway) and Murieta Drive. The Murieta Gardens I phase will consist of roughly 36.5 AC of commercial development that will include a hotel, an extended stay, commercial shops/pads, potential restaurants, one acre park, a self-storage facility and a 5.4 AC detention basin area. The Murieta Gardens II phase will consist of 78 single family residential homes on roughly 16.4 acres. Estimated wastewater production and recycled water demand for Murieta Gardens are 71.9 and 30.5 AFY, respectively. These values as well as the others described in this section were obtained from the latest developer submitted sewer studies.

The hotel is currently under construction and is expected to be completed Spring 2017. Construction of the other development phases and components are scheduled to be completed by Fall 2018.

2.3.2: The Retreats (Phase 1)

As described in the May 3 and 6, 2016 Sewer Studies, The Retreats is a Phase 1 development consisting of the following three elements located near the intersection of De La Cruz Drive and Murieta Parkway:

Retreats West: 22 single family residential homes
 Retreats North: 52 single family residential homes
 Retreats East: 10 single family residential homes

Total estimated wastewater production and recycled water demand for The Retreats (North, West, and South) are 19.8 and 15.1 AFY, respectively.

The Retreats West is currently under construction and is expected to be served with potable water for irrigation purposes on an interim basis until recycled water is available. Construction of the Retreats North and East are scheduled to be completed by Fall 2018.

2.3.3: Village A (Buildout)

Development densities for Villages A through H are based on the Preliminary Sewer Study for Rancho Murieta North. These densities are undergoing further evaluation and revision that will more likely result in lower densities.

Village A will encompass approximately 94.5 acres of which 59.0 acres are proposed for the development of 167 residential homes. This Buildout development is scheduled to receive recycled water. Estimated wastewater production and recycled water demand for Village A are 39.3 and 61.4 AFY, respectively.

2.3.4: Village B (Buildout)

Village B will encompass approximately 81.7 acres of which 63.8 acres are proposed for the development of 167 residential homes. This Buildout development is scheduled to receive recycled water. Estimated wastewater production and recycled water demand for Village B are 39.3 and 64.6 AFY, respectively.

2.3.5: Village C (Buildout)

Village C will encompass approximately 63.3 acres of which 40.8 acres are proposed for the development of 130 residential homes. This Buildout development is scheduled to receive recycled water. Estimated wastewater production and recycled water demand for Village C are 30.6 and 49.6 AFY, respectively.

2.3.6: Village D (Buildout)

Village D will encompass approximately 28.5 acres of which 24.7 acres are proposed for the development of 42 residential homes. This Buildout development is NOT currently scheduled to receive recycled water. Estimated wastewater production for Village D is 9.9 AFY.

2.3.7: Village E (Buildout)

Village E will encompass approximately 79.0 acres of which 6.3 acres are proposed for the development of 43 residential homes. This Buildout development is NOT currently scheduled to receive recycled water. Estimated wastewater production for Village E is 10.1 AFY.

2.3.8: Village F (Buildout)

Village F will encompass approximately 77.1 acres of which 36.8 acres are proposed for the development of 95 residential homes. This Buildout development is NOT currently scheduled to receive recycled water. Estimated wastewater production for Village F is 15.3 AFY.

2.3.9: Village G (Buildout)

Village G will encompass approximately 114.6 acres of which 28.7 acres are proposed for the development of 53 residential homes. This Buildout development is NOT currently scheduled to receive recycled water. Estimated wastewater production for Village G is 12.5 AFY.

2.3.10: Village H (Buildout)

Village H will encompass approximately 67.6 acres of which 49.5 acres are proposed for the development of 122 residential homes. This Buildout development is NOT currently scheduled to receive recycled water. Estimated wastewater production for Village H is 28.7 AFY.

2.3.11: Apartments (Buildout)

The Apartments will be located just east of the intersection of Highway 16 and Murieta Parkway. The Apartments encompass approximately 17.8 acres proposed for the development of 170 residential units. This Buildout development is scheduled to receive recycled water. Estimated wastewater production and recycled water demand for the Apartments are 23.3 and 23.8 AFY, respectively.

2.3.12: Residences of Murieta Hills (Buildout)

The Residences at Murieta Hills will be located in the northwest corner of the service area. This development will encompass approximately 146.1 acres of which 74.4 acres are proposed for the development of 198 residential homes. This Buildout development is scheduled to receive recycled water. Estimated wastewater production and recycled water demand for Residences of Murieta Hills are 46.6 and 73.8 AFY, respectively.

2.3.13: Lakeview (Buildout)

The Lakeview subdivision will be located in Rancho Murieta South, just west of Lakes 10 and 11. It encompasses approximately 41.6 acres proposed for development of 99 residential homes. This Buildout development is NOT currently scheduled to receive recycled water. Estimated wastewater production for Lakeview is 21.4 AFY.

2.3.14: Riverview (Buildout)

The Riverview subdivision will be located in Rancho Murieta South, just east of Lakes 10 and 11. It encompasses approximately 57.4 acres proposed for development of 140 residential homes. This Buildout development is NOT currently scheduled to receive recycled water. Estimated wastewater production for Lakeview is 32.9 AFY.

2.3.15: Industrial/Commercial/Residential (Buildout)

This development consists of a 40 acre undeveloped commercial site located on the south side of Highway 16 just west of the District's WWTP. The proposed specific uses for this site have not been determined by the developer at this time. However, according to the Preliminary Sewer Study for Rancho Murieta North, the sewer demand for the 40 acre development is anticipated to be equivalent to approximately 160 residential units, which is the value used for the development of this PDR.

2.4: Wastewater Production and Recycled Water Demand Estimates

Recycled water is produced through treatment of the community's wastewater at the District's WWTF and WWRP. Existing recycled water use within the community is currently limited to irrigation of the North and South Golf Courses and during above average levels of annual precipitation, the Van Vleck Ranch Sprayfield. Historical raw and recycled water deliveries for the North and South Golf Courses and Van Vleck Ranch Sprayfield are summarized in Table 3 and Table 4. As indicated, current and future golf course recycled water deliveries for a typical year (i.e., average levels of precipitation) are estimated to be about 550 AFY.

Table 3. Historic and Pro	piected Recycled Water	North and South Gol	lf Course Demands
Table of Thistoric and Tre	Jeeced Heey cred Water	Tion on and boatin do	1 dourse Domanas

Golf	Historic Golf	Recycled	Max Month /	•	
Course	Course Irrigation Demand (AFY)	Water Supply (AFY)	Max Day Demand (MGD)	8-hr Irrigation (gpm) ^d	9-hr Irrigation (gpm) ^e
North			1.01 ^c	2,105	1,870
South	640^a	$550^{\rm b}$	0.92c	1,915	1,705
Total			1.93	4,020	3,575

 $^{^{\}mathrm{a}}$ Average of historic 2007through 2015 golf course irrigation demands (raw plus recycled water deliveries) shown in Table 4

Table 4. Historic Golf Course and Van Vleck Ranch Water Deliveries

	Golf Course Deli	Deliveries to Van Vleck Ranch		
Year	Historic Golf Course Irrigation Demand (AFY)	Deliveries Recycled Water (AFY) ^c	(AFY) b,c	
2007	561.4	586.1	104.8	
2008	596.5	487.9	18.2	
2009	644.6	451.4	25.1	
2010	556.4	418.2	70.7	
2011	562.9	335.5	134.1	
2012	681.3	416.3	1.6	
2013	754.2	435.3	0.0	
2014	708.4	390.2	0.0	
2015	676.5	329.0	10.4	
Average	640	430	40	
Maximum	755	585	135	
Minimum	555	330	0	

^a Raw and recycled water deliveries.

^b As described in the District's WDR

^c Derived from historic records and discussed with RMCC

^d Daily 8 hour irrigation period

e Daily 9 hour irrigation period

^b Limited to 215 AFY and permitted either as part of the District's current WDR or NPDES Order No. R5-2007-0109 prior to 2015.

c. Recycled water deliveries.

Wastewater production estimates shown in Table 5 and illustrated in Figure 6 are based on the development timelines and projections previously described, and 210 gallons per day per residential home connection (gpd/connection) unit flow factor. Recycled water demand estimates were obtained from the latest information; either developer submitted sewer studies¹¹ or the Title XVI Recycled Water Feasibility Study¹².

North Maingate, Stonehouse and Escuela Parks and the District office reuse areas shown in Table 5 reflect conversion from potable to recycled water. Wastewater production shown in Table 5 for these areas is already included as part of a previous line item and thus wastewater production estimates for these particular conversions have been set to 0.

At Buildout, projected wastewater production, based on average levels of precipitation and evaporation, is estimated to be limited to about 885 AFY, which is roughly 85 AFY less than the sum of the projected recycled water demands of 970 AFY. Of this amount, the North and South Golf Courses have the highest priority for recycled water service. The total combined disposal capacity (irrigation demand) of the existing and proposed recycled water use areas, including Van Vleck, is 1,340 AFY. However, this amount of disposal capacity is only anticipated to be required following periods of unusually high levels of precipitation (e.g., 100-yr level of annual precipitation).

2.5: Design Criteria

The following are criteria that will serve as the basis for the development of the District's recommended Recycled Water Program.

2.5.1: Historic Golf Course Irrigation Demands

Historic North and South Golf Course irrigation demands were obtained from District staff and reviewed. As shown in Figure 7, the overall average irrigation demand for the last nine years (i.e., 2007 through 2016) was about 630 AFY (640 AFY without 2006 as indicated in Table 4). The average golf course irrigation demand for the last 4 years was 705 AFY or 12 percent higher than the average of the last 10 years due primarily to the affects and impacts of the recent drought. The District's WDR provides for an estimated total combined golf course recycled water demand of 550 AFY.

Monthly trends were also reviewed and are shown in Figures 8 and 9. Monthly recycled water demands in terms of AF per month are presented graphically in Figure 8 with each point representing the average of two irrigation seasons. As expected, demands are highest during the summer months due to the hotter, drier weather conditions. Monthly recycled water demands presented as percentages of the total annual irrigation season demands are shown in Figure 9. The peak month irrigation demand of 40 percent shown in Figure 9 is considered abnormal given that (1) this value is much higher compared to the others and (2) it was not repeated and thus historic peak monthly demands are expected to represent 20 to 25 percent of the annual irrigation demand. This 20 to 25 percent derived from review of historic data is slightly lower than the 31 percent described in the District's Recycled Water Standards. Discussions with District staff indicated their preference to continue to use 31% as the basis for maximum month/peak day demands.

1

¹¹ Limited to Murieta Gardens and The Retreats for both wastewater production and recycled water demand estimates.

¹²Title XVI Recycled Water Feasibility Study, Table 2-1 (p. 2-5) and Table 5-1 (p. 5-3).

¹³ See Provision 17 of the District's Waste Discharge Requirements.

 $^{^{14}\,\}mbox{See}$ Article 2.1.1 of the District's Recycled Water Standards.

Table 5. Existing and Proposed Recycled Water Production and Demand Projections

Existing Recycled Water Use Areas Existing Development Exameho Murieta North & South Golf Courses 18-hole golf courses (~250 ac) 550 380.9 Van Vleck Ranch Field 1 (~49ac), Field 2 (~25ac), Field 3 (~22 ac) 215 Sub Total 550*/765** 380 Phase 1 Proposed Expanded Recycled Water Use Areas (~2016-2020) 500*/765** 380 Main Morthgate Conversion to recycled water 2.8 0.0 Battrict Office* Conversion to recycled water 5.4 0.0 Retreats (North, East and West) 84 residential units 15.1 19.8 Murieta Gardens 78 residential units 30.5 71.9 Stonehouse Park (4-acre park) Conversion to recycled water 36.2 0.0 Escuela Park (4-acre park) Conversion to recycled water 36.2 0.0 Escuela Park (4-acre park) Conversion to recycled water 12.1 0.0 Potential conversion to recycled water 12.1 0.0 Potential conversion to recycled water 12.1 0.0 Potential conversion to recycled	Development/Proposed Recycled Water Use Area	Description	Projected RW Demand (AFY)	Wastewater Production (AFY)	
Rancho Murieta North & South Golf Courses 18-hole golf courses (~250 ac) 550 380.9 Van Vleck Ranch Field 1 (~49ac), Field 2 (~25ac), Field 3 (~22 ac) 215 Sub Total 550* / 765** 380 Phase 1 Proposed Expanded Recycled Water Use Areas (~2016-2020) Unfill 0.05 MGD allocation assumed 0 56.0 Main Northgate Conversion to recycled water 2.8 0.0 District Office* Conversion to recycled water 5.4 0.0 Retreats (North, East and West) 84 residential units 15.1 19.8 Murieta Gardens 78 residential units 30.5 71.9 Stonehouse Park (4-acre park) Conversion to recycled water 36.2 0.0 Escuela Park (4-acre park) Conversion to recycled water 36.2 0.0 Escuela Park (4-acre park) Conversion to recycled water 12.1 0.0 Commercial Loop (to be developed) Potential conversion to recycled water 56.5 35.0 Phase 2 Proposed Expanded Recycled Water Use Areas (~2020-2025) 50.0 50.0 50.0 50.0 50.0 50.0 <td< th=""><th>Existing Recycled Water Use Areas</th><th></th><th>Ţ</th><th>, ,</th></td<>	Existing Recycled Water Use Areas		Ţ	, ,	
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Murieta Gardens 78 residential units, commercial equivalent to 227 residential units 30.5 71.9 Stonehouse Park (4-acre park) Conversion to recycled water 36.2 0.0 Escuela Park (4-acre park) Conversion to recycled water 12.1 0.0 Potential conversion to recycled water 12.1 0.0 Potential conversion to recycled water Use Potential conversion to recycled water Use Proposed 50 148 Express 1 Sub Total 650* / 865** 30 Sub Total 650* / 865** 30 Phase 2 Proposed Expanded Recycled Water Use Areas (~2020-2025) Village A 167 residential units 56.5 39.3 Village B 167 residential units 64.6 39.3 Village C 130 residential units 0 9.9 Village F 95 residential units 0 10.1 Village G 53 residential units 0 22.3 Village H 122 residential units 0 25. Village G 53 residentia	District Office ^a	Conversion to recycled water	5.4	0.0	
Stonehouse Park (4-acre park) Conversion to recycled water 36.2 0.0	Retreats (North, East and West)	84 residential units	15.1	19.8	
Conversion to recycled water 12.1 0.0			30.5	71.9	
Potential conversion to recycled water; could be 20 to 30 AFY demand; require coordination with Owner to proceed Phase 1 Sub Total 650* 865* 530 Phase 2 Proposed Expanded Recycled Water Use Areas (~2020-2025) Village A 167 residential units 56.5 39.3 Village B 167 residential units 64.6 39.3 Village C 130 residential units 0 9.9 Village D 42 residential units 0 9.9 Village E 43 residential units 0 10.1 Village F 95 residential units 0 12.3 Village G 53 residential units 0 22.3 Village G 53 residential units 0 22.3 Village H 122 residential units 0 28.7 Riverview 140 residential units 0 28.7 Riverview 99 residential units 0 21.4 Apartments 170 residential units 2 3.8 23.3 Residences of Murieta Hills 198 residential units 50.9 37.6 Van Vleck Ranch Sprayfield 4 410	Stonehouse Park (4-acre park)	Conversion to recycled water	36.2	0.0	
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Sub Total 650*/865** 530 Phase 2 Proposed Expanded Recycled Water Use Areas (~2020-2025) Village A 167 residential units 56.5 39.3 Village B 167 residential units 64.6 39.3 Village C 130 residential units 49.6 30.6 Village D 42 residential units 0 9.9 Village E 43 residential units 0 10.1 Village G 53 residential units 0 22.3 Village H 122 residential units 0 28.7 Riverview 140 residential units 0 32.9 Lakeview 99 residential units 0 21.4 Apartments 170 residential units 0 21.4 Apartments 170 residential units 73.8 46.6 Industrial/Commercial/Residential 160 equivalent residential units 50.9 37.6 Van Vleck Ranch Sprayfield 4 410	Commercial Loop (to be developed)	water; could be 20 to 30 AFY demand; require coordination with Owner to			
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Apartments 170 residential units 23.8 23.3 Residences of Murieta Hills 198 residential units 73.8 46.6 Industrial/Commercial/Residential 160 equivalent residential units 50.9 37.6 Van Vleck Ranch Sprayfield 4 410 Phase 2 Sub Total 320* / 730** 355	Village C Village D Village E Village F Village G	130 residential units 42 residential units 43 residential units 95 residential units 53 residential units	49.6 0 0 0 0	30.6 9.9 10.1 22.3 12.5	
Residences of Murieta Hills 198 residential units 73.8 46.6 Industrial/Commercial/Residential 160 equivalent residential units 50.9 37.6 Van Vleck Ranch Sprayfield 4 410 Phase 2 Sub Total 320* / 730** 355	Village C Village D Village E Village F Village G Village H Riverview	130 residential units 42 residential units 43 residential units 95 residential units 53 residential units 122 residential units 140 residential units	49.6 0 0 0 0 0	30.6 9.9 10.1 22.3 12.5 28.7 32.9	
Industrial/Commercial/Residential160 equivalent residential units50.937.6Van Vleck RanchSprayfield 4410Phase 2 Sub Total320* / 730**355	Village C Village D Village E Village F Village G Village H Riverview Lakeview	130 residential units 42 residential units 43 residential units 95 residential units 53 residential units 122 residential units 140 residential units 99 residential units	49.6 0 0 0 0 0 0 0	30.6 9.9 10.1 22.3 12.5 28.7 32.9 21.4	
Van Vleck Ranch Sprayfield 4 410 Phase 2 Sub Total 320* / 730** 355	Village C Village D Village E Village F Village G Village H Riverview Lakeview Apartments	130 residential units 42 residential units 43 residential units 95 residential units 53 residential units 122 residential units 140 residential units 99 residential units	49.6 0 0 0 0 0 0 0 0 23.8	30.6 9.9 10.1 22.3 12.5 28.7 32.9 21.4 23.3	
Phase 2 Sub Total 320* / 730** 355	Village C Village D Village E Village F Village G Village H Riverview Lakeview Apartments	130 residential units 42 residential units 43 residential units 95 residential units 53 residential units 122 residential units 140 residential units 99 residential units	49.6 0 0 0 0 0 0 0 0 23.8	30.6 9.9 10.1 22.3 12.5 28.7 32.9 21.4 23.3	
·	Village C Village D Village E Village F Village G Village H Riverview Lakeview Apartments Residences of Murieta Hills	130 residential units 42 residential units 43 residential units 95 residential units 53 residential units 122 residential units 140 residential units 99 residential units 170 residential units 198 residential units	49.6 0 0 0 0 0 0 0 0 23.8 73.8	30.6 9.9 10.1 22.3 12.5 28.7 32.9 21.4 23.3 46.6	
Grand Total 970* / 1,595** 885	Village C Village D Village E Village F Village G Village H Riverview Lakeview Apartments Residences of Murieta Hills Industrial/Commercial/Residential	130 residential units 42 residential units 43 residential units 95 residential units 53 residential units 122 residential units 140 residential units 99 residential units 170 residential units 198 residential units	49.6 0 0 0 0 0 0 0 0 23.8 73.8 50.9	30.6 9.9 10.1 22.3 12.5 28.7 32.9 21.4 23.3 46.6	
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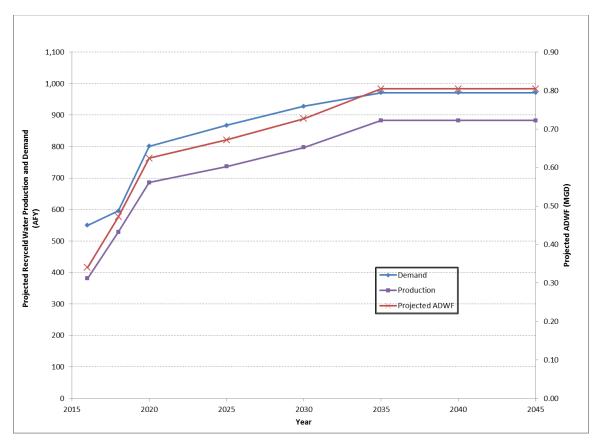


Figure 6. Recycled Water Production and Demand Estimates

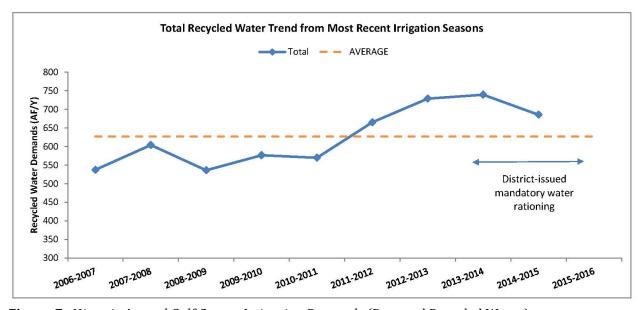


Figure 7. Historic Annual Golf Course Irrigation Demands (Raw and Recycled Water)

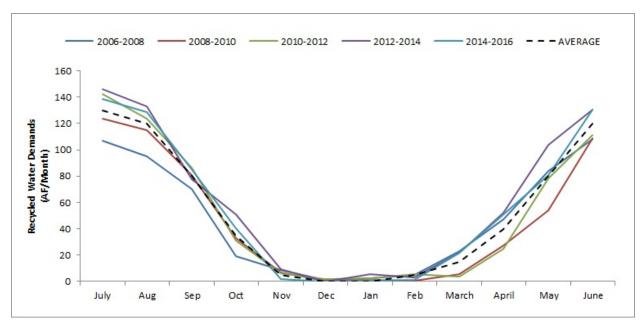


Figure 8. Historic Monthly Golf Course Recycled Water Irrigation Demands (AF per Month)

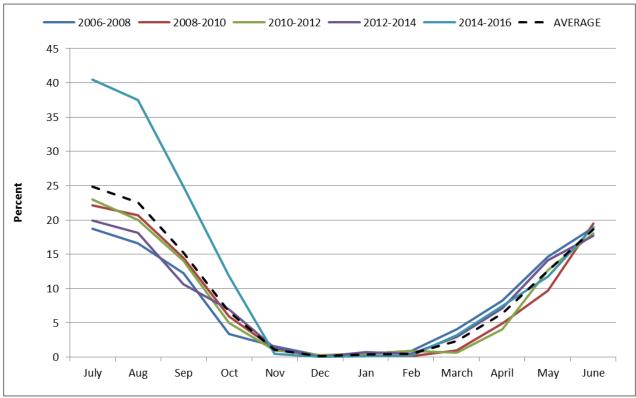


Figure 9. Historic Monthly Golf Course Recycled Water Irrigation Demands (Percent of Total Annual Demand)

2.6: Wastewater Treatment Facility and Reclamation Plant

The existing WWRP receives domestic wastewater and a relatively small amount of commercial wastewater from the community of Rancho Murieta as well as recreational vehicles (RVs) sewage

from two RV dump stations. There are no industries or industrial activities that discharge wastewater to the WWRP.

Raw wastewater is pumped to the WWTF and WWRP through three main pumping stations located throughout Rancho Murieta. The WWTF and WWRP provide secondary and tertiary treatment suitable for the production of *disinfected tertiary recycled water* as defined by Title 22 of the California Code of Regulations. Treatment processes and their locations are shown in Figure 10.

The secondary wastewater treatment plant has a permitted ADWF capacity of 1.55 MGD and a 3.0 MGD peak wet weather flow capacity. Secondary treatment takes place in a series of five clay-lined aerated facultative ponds (Aeration Ponds 1 through 5). Secondary effluent is stored in two clay-lined storage reservoirs (Reservoirs 1 and 2) with a combined storage capacity of approximately 747 AF, with two feet of freeboard, prior to tertiary treatment and disinfection. Wastewater is stored in the reservoirs during the rainy season (typically between the months of mid to late October and March) until needed for irrigation of the golf courses during the dry season. Tertiary treatment and disinfection, typically operated from April through mid-October, consists of two dissolved air floatation units, two rapid sand filters, a chlorine gas feed system, chlorine contact basin, and 6,600 linear feet of chlorine contact pipe installed in a concrete lined equalization basin. The design capacity of the tertiary treatment plant is 3.0 MGD, however the disinfection system (i.e., modal contact time) currently has a rated capacity of only 2.3 MGD. After going through tertiary and disinfection facilities, the final effluent is stored in the equalization basin prior to reuse.



Figure 10. Existing WWTF and WWRP

The existing WWTF, WWRP, and recycled water conveyance system serving the North Golf Course are to be leveraged to reduce costs associated with the Phase 1 and Buildout Recycled Water Program. The existing WWRP is designed to produce up to 3.0 MGD provided that the modal contact time is increased through the implementation of a future chlorine contact basin improvement and/or some other means as described in Section 3. The existing Recycled Water Pump Station, which draws recycled water from the equalization basin, requires expansion to satisfy projected increased recycled water demands and pressure requirements. Moreover, this station currently serves two purposes, to pump recycled water to either the North Golf Course and/or the Van Vleck Ranch Sprayfield. To maximize long term pumping efficiency and minimize costs, it is recommended that these two requirements be served by two separate pump stations in the future, if sufficient funding is available as described later in this report.

2.7: Recycled Water Use Areas and Conveyance Systems

The District produces and distributes *disinfected tertiary recycled water* to the Rancho Murieta Country Club (RMCC) for subsequent use via irrigation of two 18-hole golf course properties, the North and South Golf Courses (250 acres combined area). Both golf courses are operated by the RMCC. The locations of these golf courses are shown in Figure 11. Recycled water is pumped to the golf courses and stored in five unlined irrigation storage reservoirs (Lake 10, Lake 11, Lake 16, Lake 17, and Bass Lake) situated around the golf courses prior to beneficial reuse. The two golf courses are expected to have a combined total annual recycled water irrigation demand of 550 AF during a typical year (e.g., average levels of precipitation) as described in the District's WDR.

Disinfected tertiary recycled water can also be used to irrigate three separate pasture lands (sprayfields) on the Van Vleck Ranch. However, the District limits Van Vleck recycled water deliveries to those following wet seasons with above average levels of precipitation because those deliveries do not offset potable water demands. Distribution and use of recycled water at the Van Vleck Ranch is managed by the District. The approximate locations of Sprayfield 1 (49 ac), Sprayfield 2 (25 ac), and Sprayfield 3 (22 ac) are shown on Figure 11. The existing Van Vleck Ranch Sprayfields have a combined total irrigation demand of 215 AFY. An above ground and mobile spray irrigation system is used to apply the recycled water to the sprayfields. A similar system is assumed to be installed to accommodate future development requirements associated with above average levels of precipitation.

The following sections describe the conveyance systems associated with the golf courses and Van Vleck Ranch Sprayfields.

2.7.1: North and South Golf Courses

Recycled water conveyance and transmission systems associated with the two golf courses were installed in approximately 1983. Since that time, recycled water has been successfully used in accordance with regulatory requirements to meet golf course irrigation demands. Tertiary treated recycled water is pumped from the equalization basin located at the WWRP to Bass Lake by the Recycled Water Pump Station, which is located adjacent to the equalization basin. Recycled water to be delivered to the North Golf Course is conveyed through a 12-inch asbestos cement pipe (ACP) from the WWRP, across Highway 16, over the foot bridge (Yellow Bridge), to the 10th hole of the North Golf Course. From this point, the pipeline is reduced to an 8-inch ACP and runs east along the golf course fairways to Bass Lake. The exact alignment and/or location of this pipeline appears to be unknown at this time, as does its depth and condition.

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¹⁵ Considering construction, operating and maintenance related (e.g., net present worth) items.

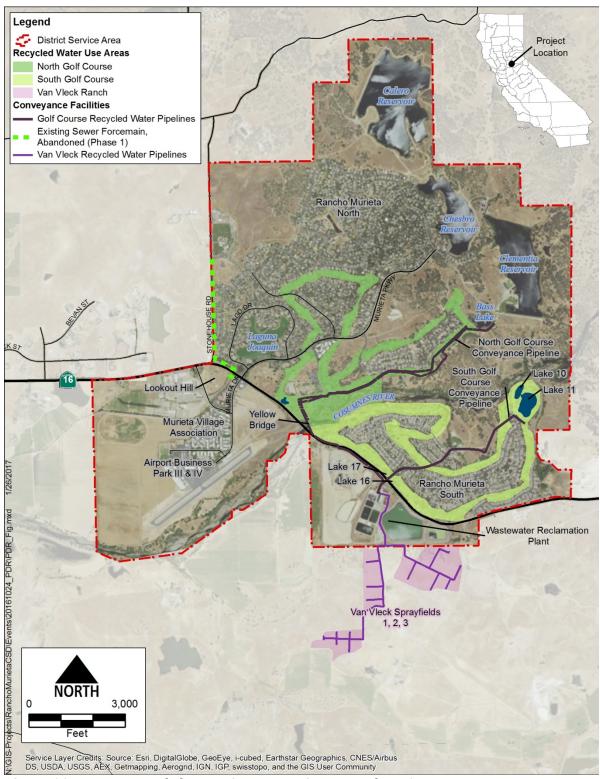


Figure 11. Existing Recycled Water Conveyance Systems and Use Areas

Tertiary treated recycled water is also conveyed by gravity from the WWRP to Lake 16 of the South Golf Course through another 12-inch ACP pipeline. Lakes 16 and 17 of the South Golf Course are interconnected by a culvert. From these lakes, recycled water is pumped to Lakes 10 and 11. The pipeline from Lake 17 to Lake 11 also runs along the golf course fairways and is 8-inch, Class 150 ACP.

Irrigation pump stations are located adjacent to both Bass Lake and Lake 11 and are controlled and operated by the RMCC. These stations continuously pump recycled water from the lakes and pressurize the golf course irrigation systems. Multiple pumps are used to meet varying demands, and fertilizer injection systems are also present. The piping material for the irrigation system is PVC and varies in size from 2- to 6-inch in diameter. The main irrigation distribution pipelines run along the golf course fairways with branches for the sprinkler heads. Irrigation valves are located throughout the golf courses to control the operation of the sprinkler heads. Most valves in the fairways control 3 to 4 sprinklers, while each sprinkler on the greens is generally controlled by individual control valves.

Table 6 presents a summary of roles and responsibilities for specific recycled water conveyance system assets. This table was derived from the Agreement for Availability and Use of Reclaimed Water (May 17, 1988) and the Amendment to Agreement for Availability and Use of Reclaimed Water (May 4, 1994).

Table 6. Recycled Water Conveyance System Roles and Responsibilities^a

System and Facility	Own	ership and O&M C	osts
	District	RMPI ^b	RMCC
Equalization Basin	X		
South	Golf Course		
Equalization Basin – Lakes 16 & 17 Pipeline	С		С
Lake 16 & 17 - Lake 10 & 11 Pipeline		D	d
North	n Golf Course		
Recycled Water Pump Station	С		С
North Golf Course Force Main to Yellow	С	С	
Bridge Pipeline			
North Golf Course Force Main from Yellow		Е	e
Bridge to Bass Lake Pipeline			

^a Adapted from Agreement for Availability and Use of Reclaimed Water (May 17, 1988) and the Amendment to Agreement for Availability and Use of Reclaimed Water (May 4, 1994)

2.7.2: Van Vleck Ranch Pipelines

Recycled water can also be pumped from the existing Recycled Water Pump Station to Van Vleck Ranch. Typically, this is only done during years of above average levels of precipitation but is also done at least once every two years to maintain the associated easement rights. Recycled water can be transmitted to Van Vleck Ranch through approximately 1,800 linear feet of aboveground piping. Both 12- and 8-inch Certa-Lok™ PVC irrigation pipes are used to convey recycled water to the Van Vleck Ranch boundary, and about 4,050 linear feet (LF) of aboveground 8-, 6-, 4-, and 3-inch Certa-Lok™ PVC irrigation pipe is used to convey recycled water to three spray irrigation systems. The 12- and 8-inch PVC pipeline was installed in 2007 and is owned and operated by the District. One of the

^b Rancho Murieta Properties, Inc. (RMPI) was the original owner, current owner is Rancho North Properties, LLC.

c RMCSD to own, operate and maintain; operation and maintenance costs to be split 50/50 between RMCSD and RMCC.

 $^{^{}m d}$ RMPI to own, RMCC to operate and maintain; operation and maintenance costs to be split 50/50 between RMPI and RMCC.

^e RMPI to own, RMCC to operate and maintain.

three existing pumps within the Recycled Water Pump Station is used to convey recycled water through the transmission pipeline to three sprayfields. There are no potable water or sewer pipelines along the transmission or distribution pipeline alignment.

The distribution system consists of approximately 29 strings of K-line irrigation systems, which are in turn composed of movable sprinklers and 40 millimeter (mm) piping. Each movable sprinkler is housed within a plastic pod. The connecting piping is flexible and the entire string of sprinklers can be moved within each sprayfield.

2.7.3: Existing 12-inch Sewer Force Main

As described in the District's Initial Study/Mitigated Negative Declaration (AECOM, June 2014), an existing 12-inch ACP sewer force main may be used in some fashion to convey recycled water to Stonehouse Park (Phase 1), Escuela Park (Phase 1) and Residences of Murieta Hills (Buildout) for recycled water irrigation. As shown in Figure 11, this pipeline extends from the District's Main Lift North Pumping Station to the Stonehouse Park. The District is currently conducting a condition assessment of this pipeline to determine how best to leverage this asset in the future. Future condition assessment is expected to be conducted for the 8- and 12-inch ACPs that convey recycled water from the WWRP to Bass Lake. Information drawn from this Phase 2 assessment will be helpful in refining costs for rehabilitating the 12-inch sewer force main.

Preliminary pipeline risk assessment results developed by Kennedy/Jenks predict the 12-inch sewer force main (FM) to fall within the "moderate range of likelihood of failure" range. However, because of the aggressive recycled water quality, the pipeline has been elevated into the "High Range". The recommended action plan for the High Range is a "proactive and detailed assessment of the pipe", which justifies the rehabilitation plan currently underway for all existing recycled water pipelines.

Considering both cost, access and constructability, the recommended rehabilitation technique is a thin wall, non-structural liner solution in locations where the 12-inch sewer force main is deemed to be structurally viable. The renovation techniques anticipated and included within the cost estimate described later in the PDR for other areas of the force main are either a spray on cement lining, or a cure-in-place liner solution. Both techniques are similar in cost per linear foot and provide similar rehabilitation to the pipe.

Where a structural solution is deemed necessary, the renovation/replacement techniques anticipated and included within the cost estimate are either open-cut pipe replacement or a pull in pipe-bursting solution. Open-cut pipe replacement, considered the more expensive of the two options in terms of cost per linear foot, is the technique included in the cost estimate at this time.

2.8: Conveyance System Requirements

The hydraulic model developed by AECOM was updated and modified to reflect the proposed configuration of the Buildout recycled water system and setup to provide separate irrigation cycles to accommodate golf course and urban and residential recycled water demands. The model and other data sources (i.e., drawings) served as the means of determining the conveyance system operating requirements, limitations, etc. described below in Sections 2.8.1 through 2.8.5.

2.8.1: Recycled Water Supplies and Demands

Recycled water demands shown in the draft AECOM hydraulic model were adjusted to reflect those described in this PDR. Supplies were limited to the production from the WWRP. Tanks and golf course lakes were used to provide operational storage to help satisfy diurnal and instantaneous demands. Demands were limited to existing and proposed reuse areas.

2.8.2: Pressure Limitations of Existing Pipelines

The District's ability to convey recycled water both now and in the future relies heavily upon existing Class 150 ACP pipelines, which are close to 33 years old and have rated pressure limitations of about 150 pounds per square inch (psi). The updated model was configured to limit pipeline pressures to below this limitation by:

- Adding a pressure reducing valve (PRV) immediately downstream of the proposed Recycled Water Pumping Station. The downstream PRV setting was 150 psi.
- Verifying that the modeled pressures in the entire system do not exceed the maximum operations pressure of 150 psi.

2.8.3: Recycled Water Tank Locations and Elevations

The proposed Lookout Hill Recycled Water Tank was assumed to be configured relatively the same as the abandoned existing tank with respect to size, elevation and maximum water level as assumed and described in the District's Initial Study/Mitigated Negative Declaration.

The location of the Bass Lake Tank was also reviewed using the updated hydraulic model. Modeling results indicate that:

- Bass Lake Tank should be located at an elevation that will maintain the Hydraulic Grade Line (HGL) in the existing 8-inch ACP pipeline above the topography's high point to avoid negative pressures in the pipeline; the tank should be set at a base elevation of at least 225.
- Bass Lank Tank should be located relatively close to the existing 8-inch ACP pipeline and uphill, where elevations are increasing (as opposed to on the downside of a hill).

The following are summaries of recommended tank criteria to be used for developing preliminary layouts and costs:

 Table 7. Recycled Water Storage Tank Design Criteria (Preliminary)

Recommend Criteria / Requirements	Lookout Hill Tank	Bass Lake Tank
Number of Tanks	1	1
Nominal Volume, gal	200,000	500,000
Diameter, ft	40	70
Working Depth, ft	4 to 22	4 to 18
Tank Base Elevation	244	<u>≥</u> 225

2.8.4: System Controls

The use of the existing ACP conveyance pipelines and their associated hydraulic capacities, limitations, etc. dictate the need to replenish golf course lakes separately from urban and residential recycled water irrigation demands with respect to time. It has been assumed that urban and residential irrigation will occur over an 8- or 9-hour period between the hours of 9 or 10 pm and 6 am. The refilling of the golf course lakes will take place between the hours of 6 am and 9 or 10 pm, during the periods when urban and residential irrigation are not occurring. The following is a summary of the irrigation cycle times used for system modeling:

• Urban and Residential Irrigation: 8- or 9-hour period between 9 or 10 pm and 6 am

• Refilling of Golf Course Lakes: 6 am and 9 or 10 pm (non-urban and residential

irrigation hours)

Golf Course Irrigation:
 May occur at any time and be drawn from Lakes 10,

11, 16, 17 and Bass Lake

Timing of recycled water deliveries is anticipated to require the installation of the following process, flow, etc., control elements. These items were incorporated into the hydraulic model and will serve as the basis for developing the instrumentation and control cost estimates described in Section 4.

- 1. **Recycled Water Pumping Station Pressure Reducing Valve (Phase 1).** To be located immediately downstream of proposed Recycled Water Pumping Station. Limit pressurization of pipelines to below the maximum operations pressure.
- 2. **Recycled Water Pumping Station Flow Meter (Phase 1).** To be located immediately downstream of proposed Recycled Water Pumping Station. Meter demands and records in Supervisory Control and Data Acquisition (SCADA).
- 3. **Recycled Water Pumping Station Pressure Gauge (Phase 1).** To be located downstream of proposed Recycled Water Pumping Station along existing 12-inch ACP pipeline or at critical (i.e., location experiencing highest pressure) location near Yellow Bridge.
 - Measures pipeline operating pressure and records in SCADA. The speed of the pumps within the Recycled Water Pumping Station will be reduced upon a high pressure reading or shutdown if needed.
- 4. **Lookout Hill Flow Control (Open / Close) Valve (Phase 1).** To be installed and used to isolate the 12-inch pipeline leading to Murieta Gardens (and ultimately Stonehouse and Escuela Parks and Residences of Murieta Hills) from the existing North Golf Course Transmission Main. This leg will be shut off and refilled from the Lookout Hill Tank and pressurized by the Lookout Hill Booster Pumping Station when Bass Lake is being filled for golf course irrigation.
 - This flow control valve should be configured to open based on time when urban and residential irrigation begins at 9 or 10 pm and close once urban and residential irrigation has been completed and the Lookout Hill Tank is full; which is anticipated to be around 6 am
- 5. **Bass Lake Flow Control (Open/Close) Valve (Phase 1).** To be installed to control recycled water conveyance into Bass Lake. The valve is recommended to be located on the existing Bass Lake pipeline downstream of the split to Bass Lake Tank connection. The Bass Lake fill pipeline will essentially be isolated (shut off) from the remaining system during urban and residential irrigation.
 - This flow control valve should be configured to close based on time when urban and residential irrigation begins at 9 or 10 pm and remain closed through 6 am.
- 6. **Lookout Hill Tank Altitude Valve (Phase 1).** To be installed to automatically shutoff recycled water source once the tank has reached a predetermined maximum operating level (assumed to be 266 in the hydraulic model).
- 7. **Bass Lake Tank Altitude Valve (Buildout).** To be installed to automatically shutoff recycled water source once the tank has reached a predetermined maximum operating level (assumed to be 243 in the hydraulic model).

- 8. **Lookout Hill Booster Pumping Station (Phase 1).** To be installed downstream of the proposed tank and have a nominal capacity of 1,000 gpm. In order to support the delivery of recycled water for drip irrigation throughout the day, the Lookout Hill Booster Pumping Station will be configured to maintain pressure within the 12-inch pipeline serving Murieta Gardens, Stonehouse and Escuela Parks, Main Northgate and Residences of Murieta Hills to a predetermined set point during the golf course irrigation cycle.
- 9. **Bass Lake Tank Booster Pumping Station (Buildout).** To be installed downstream of the proposed tank and have a nominal capacity of 1,200 gpm.
- 10. **Lookout Hill Pressure Gauge (Phase 1).** To be installed downstream of Lookout Hill Flow Control Valve along 12-inch pipeline, potentially at critical location (i.e., location experiencing highest pressure) near Main Lift North Pumping Station.

This pressure gauge will continuously monitor pipeline pressure and send this data to SCADA. If operating pressures above the pipeline's capacity are experienced, SCADA will lower the pump speed or shut down the Recycled Water Pumping Station pumps. In order to support the delivery of recycled water irrigation throughout the day, the Lookout Hill Booster Pumping Station will be configured to maintain pressure within the 12-inch pipeline serving Murieta Gardens, Stonehouse and Escuela Parks, Main Northgate and Residences of Murieta Hills if needed to a predetermined set point during the golf course irrigation cycle.

2.8.5: Proposed Operating Strategy

The following tables provide a summary of the proposed statuses and actions of the system elements during urban and golf course irrigation cycles.

Table 8. Proposed Strategy - Phase 1 Operations

Sys	tem Element	Urban and Residential	Golf Course Supply	
		Irrigation		
	Approximate Timeframe	9 or 10 pm to 6 am	6 am to 9 or 10 pm	
1	RWPS PRV	Measure , SCADA Monitors, ≥ 150	≥ 150 psi; lower speed, shutdown	
		psi lower speed, shutdown	pumps if required	
		pumps if required		
2	RWPS Flow Meter	Measure and Record	Measure and Record	
3	RWPS Pressure Gage	Measure, SCADA Monitor	Measure, SCADA Monitor	
4	Lookout Hill Flow Control Valve	Open	Closed	
5	Bass Lake Flow Control Valve	Closed	Open	
6	Lookout Hill Tank Altitude Valve	Open; Periodically Closed w/Fill	Closed	
7	Bass Lake Tank Altitude Valve	Future	Future	
8	Lookout Hill Booster Pumping Station	1,000 gpm @ 150 ft TDH to maintain	Configured to maintained nominal	
		minimum 40 psi to downstream	pressure	
		service		
9	Bass Lake Booster Pumping Station	Future	Future	
10	Lookout Hill Pressure Gauge	Measure, SCADA Monitors; ≥ 150 psi	Measure, SCADA Monitor; Turn on	
		shutdown Recycled Water Pumping	Lookout Hill Booster Pumping	
		Station pumps	Station on low pressure set point	

Table 9. Proposed Strategy - Buildout Operations

System Element		Urban and Residential Irrigation	Golf Course Supply	
	Approximate Timeframe	9 or 10 pm to 6 am	6 am to 9 or 10 pm	
1	RWPS PRV	Measure , SCADA Monitors, ≥ 150 psi shutdown pumps	≥ 150 psi; shutdown pumps	
2	RWPS Flow Meter	Measure and Record	Measure and Record	
3	RWPS Pressure Gage	Measure, SCADA Monitor	Measure, SCADA Monitor	
4	Lookout Hill Flow Control Valve	Open	Closed	
5	Bass Lake Flow Control Valve	Closed	Open	
6	Lookout Hill Tank Altitude Valve	Open; Periodically Closed w/Fill	Closed	
7	Bass Lake Tank Altitude Valve	Open	Open until tank filled, then Closed	
8	Lookout Hill Booster Pumping Station	1,000 gpm @ 150 ft TDH to maintain minimum 40 psi to downstream service	Configured to maintained nominal pressure notaries	
9	Bass Lake Booster Pumping Station	1,200 gpm @ 120 ft TDH to maintain minimum 40 psi to downstream service	Configured to maintained nominal pressure	
10	Lookout Hill Pressure Gauge	Measure, SCADA Monitors; ≥ 150 psi shutdown Recycled Water Pumping Station pumps	Measure, SCADA Monitor; Turn on Lookout Hill Booster Pumping Station on low pressure set point	

2.9: Regulatory Compliance

The following describe the status of the District's Recycled Water Program with respect to environmental (California Environmental Quality Act) and regulatory (Regional Board) review.

2.9.1: Environmental Compliance

The final IS/MND determined that expanding the District's recycled water areas to serve new development within the District's service area would not have any significant adverse effects on the environment based on a specific system configuration and after implementing the following mitigation measures ¹⁶:

AESTHETICS

 Mitigation Measure AES-1: Replace Landscaping. District to coordinate with affected landowners to restore or replace plantings consistent with pipeline safety, maintenance, and easement requirements in affected landscape areas.

AIR QUALITY

 Mitigation Measure AQ-1: Implement Applicable SMAQMD Basic Construction Emission Control Practices. District to comply with prescribed measures to reduce fugitive dust and construction equipment exhaust emissions.

 Mitigation Measure AQ-2: Implement SMAQMD Requirements to Reduce Construction-Related NOX Emissions. District and/or contractor to submit to SMAQMD comprehensive inventory of all off-road diesel construction equipment, equal to or greater than 50 horsepower, that will be used in aggregate of 40 or more hours during any portion of construction.

 $^{^{16}}$ Complete listing of mitigation measures is provided in this PDR along with brief descriptions. More complete descriptions and information can be obtained from the IS/MND.

BIOLOGY

- Mitigation Measure BIO-1: Protect Special-status Plant Species. District and its primary
 construction contractor shall implement prescribed measures to reduce impacts on specialstatus plant habitat.
- Mitigation Measure BIO-2: Protect Valley Elderberry Beetle. District and its primary
 construction contractor shall implement prescribed measures to reduce impacts on valley
 elderberry beetles.
- Mitigation Measure BIO-3: Protect Western Pond Turtle. District and its primary
 construction contractor shall implement Mitigation Measures HYD-1 and HYD-3 to ensure
 no construction area erosion, sedimentation, or pollution enters any western pond turtle
 habitat.
- Mitigation Measure BIO-4: Conduct Pre-Construction Surveys for Swainson's Hawk and Implement Avoidance and Minimization Measures. District and its primary construction contractor shall implement specific prescribed measures to protect nesting Swainson's hawks.
- Mitigation Measure BIO-5: Conduct Pre-Construction Surveys for Nesting Raptors and Other Migratory Birds and Implement Avoidance and Minimization Measures. District and its primary construction contractor shall implement specific prescribed measures to protect nesting raptors and other nesting migratory birds.
- **Mitigation Measure BIO-6: Worker Environmental Awareness Program.** Before start of each new construction season, a worker environmental awareness training program shall be conducted by a qualified biologist.
- **Mitigation Measure BIO-7: Protect Wetlands and Drainages.** District and its primary construction contractor shall implement specific prescribed measures to reduce impacts to wetlands and drainages.
- **Mitigation Measure BIO-8: Comply with Tree Preservation Ordinance.** District and its primary construction contractor shall implement specific prescribed measures to reduce impacts to protected oaks and other native trees.

CULTURAL RESOURCES

- Mitigation Measure CUL-1: Immediate Halt Construction Activities If Any Cultural Materials Are Discovered.
- Mitigation Measure CUL-2: Conduct Construction Personnel Education, Stop Work if Paleontological Resources Are Discovered, Assess the Significant of the Find, and Prepare and Implement a Recovery Plan Required. To minimize potential adverse impacts on important paleontological resources, District, where construction would occur along or in the immediate vicinity of Stonehouse Road, shall retain qualified paleontologist to train all construction personnel and immediately cease work in the vicinity of the find and notify the Sacramento County Planning and Community Development Department.
- Mitigation Measure CUL-3: Immediately Halt Construction Activities if Any Human Remains Are Discovered.

GEOLOGY

• Mitigation Measure GEO-1: Prepare a Site-Specific Landslide Hazard Evaluation and Implement Engineering Recommendations. District to hire licensed geotechnical or civil engineer to perform site-specific evaluation of the landslide potential in areas of moderate or steep slopes where each of the proposed storage tanks would be placed.

• Mitigation Measure GEO-2: Prepare and Implement a Grading and Erosion Control Plan. Before start of earthmoving activities greater than one acre of disturbance, District to prepare grading and erosion control plan and submit to Sacramento County Planning and Development Department for review before issuance of any grading permit for on-site work.

HAZARDS

- Mitigation Measure HAZ-1: Implement a Site Investigation to Determine the Presence of Naturally Occurring Asbestos (NOA) and, if necessary, Prepare and Implement Asbestos Dust Control Plan. District to conduct site investigation to determine whether and where NOA is present in the construction area. If site investigation determines that NOA is present within the proposed construction area then the District to prepare an Asbestos Dust Control Plan for approval by SMAQMD.
- Mitigation Measure HAZ-2: Prepare and Implement a Construction Traffic Control Plan. District and its primary construction contractor to prepare and implement traffic control plan for construction activities.

HYDROLOGY AND WATER QUALITY

- Mitigation Measure HYD-1: Prepare and Implement a Storm Water Pollution Prevent Plan and Associated Best Management Practices. For activities disturbing 1 or more acres (including phased construction of smaller areas that are part of the District's Recycled Water Program), District and its primary construction contractor to obtain coverage under the SWRCB's NPDES stormwater permit for general construction activities (Order No. 2009-0009-DWO).
- Mitigation Measure HYD-2: Evaluate and Implement Construction Site Dewatering Controls. If construction dewatering is required, District shall evaluate reasonable options for dewatering management and ensure that controls on construction site dewatering are implemented during construction dewatering activities.
- Mitigation Measure HYD-3: Prepare and Implement a Fac-Out and Undercrossing Contingency Plan. If drilling mud is needed during construction, the District will develop and follow procedures to prevent the mix that is used during drilling from being discharged onto the ground surface when installing pipelines using trenchless construction methods.

NOISE POLUTION

- Mitigation Measure NOI-1: Provide Noise Shielding for Pump Stations. District to
 design the proposed pump station with shielding, as needed, to achieve noise levels below
 55 dBA at 50 feet.
- Mitigation Measure NOI-2: Implement Feasible Noise Abatement Measure for Construction Equipment. District to require contractors to implement feasible noise abatement measures for noise-producing equipment.

RECREATION

• Mitigation Measure REC-1: Coordinate with RMCC Prior to Construction. District to coordinate with RMCC at least 30 days prior to construction activities that could affect golf course operations, including access to the course and course play.

2.9.2: Regulatory Requirements

As previously described, the District falls under the jurisdiction of the Regional Board with respect to wastewater and recycled water. A summary of specific requirements related to the District's need to provide sufficient seasonal storage capacity, approval of proposed future WWRP and recycled water system improvements and use areas are described below and were obtained from the District's WDR:

- **Seasonal Storage Capacity:** On or about 1 October of each year, available storage capacity shall at least equal the volume necessary to provide sufficient capacity to accommodate allowable wastewater flow, design seasonal precipitation, and ancillary inflow and infiltration during the winter while ensuring continuous compliance with all WDR requirements. Design seasonal precipitation shall be based on total annual precipitation using a return period of 100 years, distributed monthly in accordance with historical rainfall patterns.
- Recycled Water System Improvements and Future Recycled Water Use Areas: The District shall submit an *Improvements Completion Report* upon completion of any improvements, which may include expansion of the disinfection system, effluent storage, and/or recycled water distribution system and infrastructure improvements to deliver recycled water to the new and expanded recycled water use areas as described in the District's WDR. The *Improvements Completion Report* shall be submitted to the Regional Board for review and approval at least 60 days prior to operational use of such improvements, facilities and/or systems. The report shall document the construction of the improvements, certify that improvements are fully functional, and certify that any new or expanded recycled water use areas are ready to receive recycled water in compliance with the requirements of the District's WDR. The report shall include design parameters (for treatment system), final dimensions and volume at 2-feet of freeboard (for ponds), as-built drawings of the WWRP improvements, and a map showing new recycled water use areas.
- **WWRP:** The District shall submit a *Capacity Increase Report* documenting that the WWRP has sufficient storage and disposal capacity for increasing the WWRP ADWF influent flow to more than 0.5 MGD while being in compliance with all applicable specifications, limitations, and provisions of the District's WDR. The report shall certify that the new recycled water use areas (e.g., existing parks and common area, recycled water residential irrigation developments and/or expanded Van Vleck Ranch Use Area (Sprayfield 4)) are ready to receive recycled water in compliance with the requirements of the WDR. The *Capacity Increase Report* shall be submitted to the Regional Board for review and approval at least 60 days prior to increasing the WWRP influent flow beyond 0.5 MGD.

Section 3: Recommended Improvements

This section presents design features and descriptions for the recommended Phase 1 Recycled Water Improvements Project which is comprised of Phase 1 WWRP Improvements and Phase 1 Recycled Water Conveyance System Improvements. Recommended future Buildout Recycled Water Improvements have also been identified and recommended. The features described in the tables below were developed from the criteria described in Section 2 of this PDR. A summary of Phase 1 and Buildout Recycled Water Improvements are presented in Tables 10 and 11, respectively.

Table 10. Recommended Phase 1 Recycled Water Improvements Features and Components

Process / Element	Criteria / Feature
1. Recycled Water SCADA Control System	Griteria / Foutaro
Number of SCADA Terminals	1
Location	WWRP
Type	
Lookout Hill	Programmable Logic Controller (PLC)
Control Valves	Remote Terminal Units
Communication	Radio*
Control	Pressure
2. Equalization Basin Potable Water Air Gap Com	I.
Flow Rate (maximum)	900 gpm
Diameter	8-inch
Material	Ductile Iron
Air Gap (90° Bend)	16 inches per RW-17
3. Rehabilitate Existing Recycled Water Pumping	
Pump Type	Vertical Turbine
Number of Pumps	Two (2) duty; one (1) stand by
Total Dynamic Head	325 feet
Pump Flow	1,500 gpm
Motor Horsepower	200 HP
Backup Power	200 KW Standby Diesel Generator
Control Method	Pressure
4. District Headquarters Conversion - Recycled V	Vater Irrigation System Connection
Site Supervisor	District (Paul Siebensohn)
Type of Landscape	Grass in front yard and medians
Type of Irrigation	Spray and drip
Area (approximate)	168 acres
Water Demand (estimated)	5.4 AFY
Pipe Diameter	4-inch
Pipe Material	PVC
5. Northwest Recycled Water Transmission Main	
Pipeline Length (total)	11,600 lineal feet, total
Highway 16 Undercrossing	1,000 lineal feet (approximately)
Legacy Lane to Lookout Hill Tank	2,800 lineal feet (approximately)
Lookout Hill Tank to 12-inch Force Main	2,400 lineal feet (approximately)
12-inch Force Main along Stonehouse Road to	5,400 lineal feet (approximately)
Stonehouse and Escuela Parks	
Replace	1,200 lineal feet of 12-inch
CIPP Rehabilitation	2,400 lineal feet of 12-inch
Diameter	12 inch

Pr	ocess / Element	Criteria / Feature
	Buried Pipeline Materials	PVC or HDPE pipe
	Above Grade Pipeline Materials	Steel or Ductile Iron pipe
	Pipeline Labeling	"Recycled Water, Do Not Drink"
	Pipe Color or Wrapping	Purple or wrapped with purple tape
	Air and Blowoff Valves	District Standards
	Others	See District Standards
6.	Lookout Hill Booster Pumping Station	,
	Pump Type	Vertical Turbine
	Number of Pumps	One (1) duty; one (1) stand by
	Total Dynamic Head	150 feet TDH
	Pump Flow	1,000 gpm (maximum)
	Motor Horsepower	50 HP
	Pump Housing	Not required
	Backup Power	50 KW Standby Diesel Generator
	Control Method	Pressure
7.	Escuela Park Conversion - Recycled Water Irrig	gation System Connection
	Site Supervisor	Rancho Murieta Association (RMA) (TBD)
	Type of Landscape	Plantings and flowers now
	Type of Irrigation	Spray and drip
	Area (approximate)	4 acres
	Water Demand (estimated)	12.1 AFY
	Pipe Diameter	4-inch
	Pipe Material	PVC
8. Stonehouse Park Conversion - Recycled Water Irrigation System Connection		Irrigation System Connection
	Site Supervisor	RMA (TBD)
	Type of Landscape	Grass primarily (fields)
	Type of Irrigation	Spray and drip
	Area (approximate)	4 acres
	Water Demand (estimated)	36.2 AFY
	Pipe Diameter	4-inch
	Pipe Material	PVC
9.	Lookout Hill Recycled Water Storage Tank	
	Number of Tanks	1
	Diameter	40
	Height (maximum at sidewall)	26
	Volume (nominal)	200,000 gallons
	Materials of Constructed	Bolted Steel
10	. North Maingate Conversion - Recycled Water	Irrigation System Connection
	Site Supervisor	RMA (TBD)
	Type of Landscape	Grass, flower beds, plantings
	Type of Irrigation	Spray and drip
	Area (approximate)	121 acres
		2.0 AEV
	Water Demand (estimated)	2.8 AFY
	Water Demand (estimated) Pipe Diameter	4-inch

^{*}Wireless I/O can be used alternatively

Table 11. Recommended Buildout Recycled Water Improvements Features and Components

Table 11. Recommended Buildout Recycled Water Impl	
Process / Element	Criteria / Feature
A. Disinfection Facilities Upgrade	
Existing Contact Basin Modal Contact Time	27 minutes at 3.0 MGD ¹
Required Modal Contact Time	90 minutes (minimum)
Additional Modal Contact Time Required	63 minute (minimum)
New Contact Basin Efficiency	90%
(Assumed Baffling Factor)	
Required Contact Basin Volume	145,835 gal, minimum; 146,610 gal actual
Length to Width to Depth Ratios	Target 40:1:1.5; Actual 40:1:1.4
Length (without walls)	280 ft total (3 passes, each at 93.33 ft long)
Width (without walls)	21 ft total (3 passes, each at 7 ft wide)
Depth (without walls)	10 ft
B. North Golf Course Conveyance System Rehabilita	
WWRP to Bass Lake	11,200 lineal feet (12- and 8-inch)
Replacement	4,300 lineal feet, 12-inch
CIPP Rehabilitation	3,800 lineal feet, 8-inch
Replacement	1,900, 8-inch
C. Bass Lake Recycled Water Storage Tank	
Number of Tanks	1
Diameter	70
Height (maximum at sidewall)	22
Volume (nominal)	500,000 gallons
Materials of Constructed	Bolted Steel
D. Bass Lake Booster Pumping Station	
Pump Type	Vertical Turbine
Number of Pumps	One (1) duty; one (1) stand by
Total Dynamic Head	120 feet
Pump Flow	1,200 gpm
Motor Horsepower	50 HP
Pump Housing	Not required
Backup Power	50 KW Standby Diesel Generator
Control Method	Pressure
E. Seasonal Storage Reservoir	·
Existing Storage Capacity	728.2 AF
Required Storage Capacity (Buildout)	765 AF
Incremental Capacity Upgrade	40 AF
F. Van Vleck Sprayfield No. 4	
Extension of Recycled Water Transmission Main	1,000 lineal feet of 12-inch Certa-Loc™
Sprayfield 4 Transmission Main	5,000 lineal feet of 8-inch Certa-Loc™
Sprayfield 4 Transmission Main	5,000 lineal feet of 6-inch Certa-Loc™
Sprayfield 4 Transmission & Distribution Mains	16,250 lineal feet of 4-inch Certa-Loc™
Irrigation System	55 K-line Strings
Depth of Cover	None, all located aboveground
G. Dissolved Air Flotation Feed Pump Improvemen	-
Replacement of 3rd Feed Pump	\$100,000 Allocation
- Programme and a second	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,

¹ See Figure 1-3 of *WWRP Modified Chlorine Contact Disinfection System Compliance Report* (HSe, July 2006). Equivalent volume of 56,250 gallons

3.1: Recommended Phase 1 WWRP Improvements

The four recommended Phase 1 WWRP improvements are illustrated in Figure 12. Descriptions of each recommended improvement are provided after Figure 12.

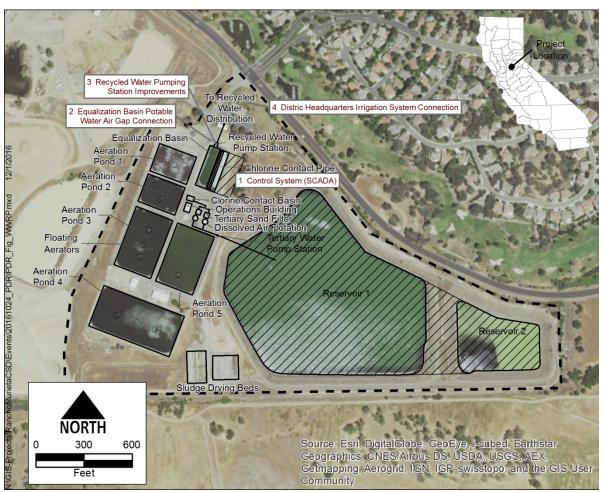


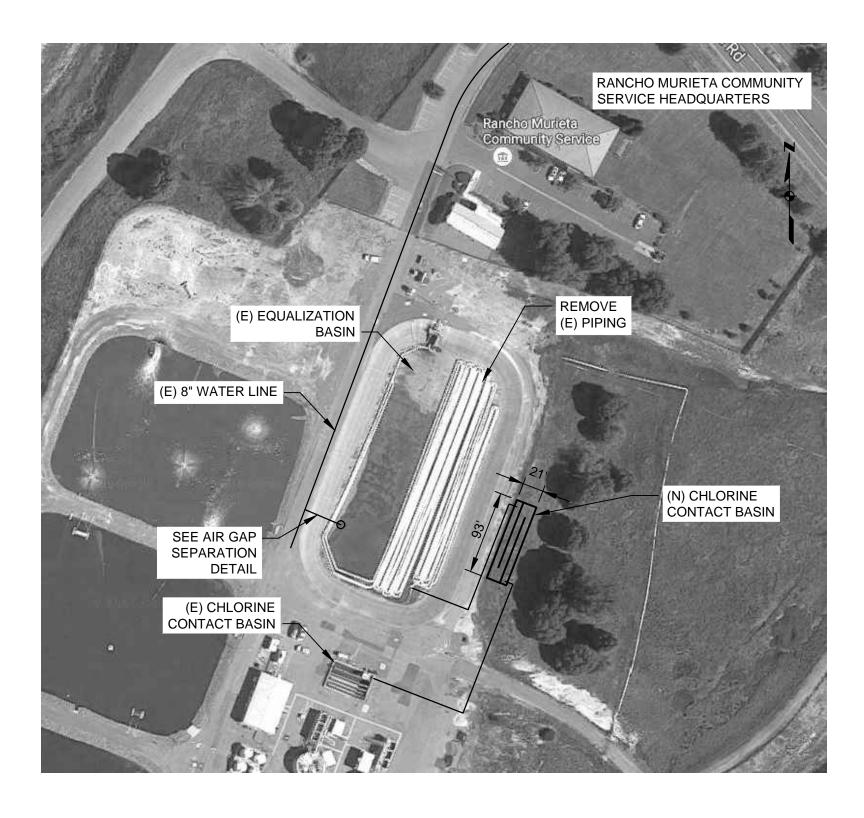
Figure 12. Proposed Phase 1 WWRP Improvements

3.1.1: Control System for Recycled Water Conveyance and Storage System

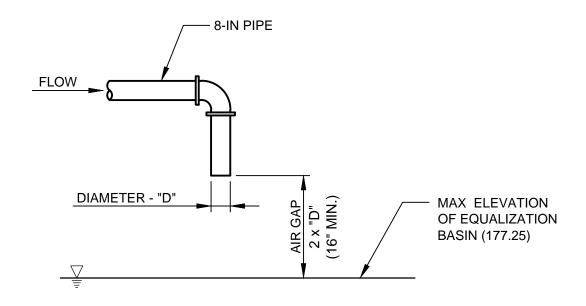
A SCADA system and telemetry is recommended to control delivery of recycled water throughout the existing and proposed recycled water conveyance and storage system. This also includes the installation of the control valves and elements previously described in Section 2.8.4 to manage and monitor recycled water storage, conveyance and distribution.

3.1.2: Equalization Basin Potable Water Air Gap Connection

This improvement is required to supplement recycled water with potable water and meet peak recycled water demands while maximizing the use of recycled water within the community. This improvement requires connection to the existing 8-inch (in) potable water pipeline located immediately north of the equalization basin at the WWRP, installing an 8-inch extension to the equalization basin, and installing an 8-in air gap connection to deliver potable water to the equalization basin. Figure 13 shows the proposed pipeline and air gap separation. The connection between the existing potable water pipeline and the air gap will require approximately 20 feet (ft) of 8-in ductile iron pipe (DIP) and a flow meter, isolation and control valves and bends. The existing 8-inch potable water pipeline is assumed to have a capacity of 900 gpm or greater.







NOTE:

1. THE TERM "AIR GAP" SHALL MEAN A PHYSICAL SEPARATION BETWEEN THE FREE FLOWING DISCHARGE END AND A RECYCLED WATER SUPPLY PIPELINE AND AN OPEN OR NON-PRESSURE RECEIVING VESSEL. AN "APPROVED AIR GAP" SHALL BE AT LEAST DOUBLE THE DIAMETER OF THE SUPPLY PIPE MEASURED VERTICALLY ABOVE THE OVERFLOW RIM OF THE VESSEL - IN NO CASE LESS THAN 1 INCH.



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RANCHO MURIETA COMMUNITY SERVICE DISTRICT

PROPOSED POTABLE WATER AIR GAP AND CHLORINE CONTACT IMPROVEMENTS

> K/J 1670011*00 DEC 2016

> > Figure 13

Recycled Water Program Preliminary Design Report (Draft)

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Projected average and maximum month/maximum day potable water supplementation requirements are summarized in Table 12:

Table 12. Projected Recycled and Supplemental Potable Water Demands^b

	Recycled Water Demands			Supplemental Potable Water Requirementsb		
Condition	Avg Annual	Max Month/Max Day	Instan Urban /	Avg Annual	Max Month/Max	Instan Urban
Condition	(AFY)a	(MGD)	Golf Course	(AFY) ^a	Day	/ Golf Course
			(gpm)		(MGD)	(gpm)
Phase 1	650	2.27	715° / 2,010d	120	0.30	900 / 310
Buildout	970	3.35	2,955c / 2,010d	110	0.35	900 / 0

^a Values rounded to the nearest 5

3.1.3: Rehabilitate Recycled Water Pumping Station

The objective of this improvement is to provide adequate pumping capabilities to the North Golf Course Transmission Main through the rehabilitation of the existing Recycled Water Pumping Station. Currently, this facility is configured to pump recycled water to either the North Golf Course or Van Vleck Ranch. Following rehabilitation, this station will continue to operate in this fashion, but with an increased firm capacity to satisfy maximum month / maximum day demands of the North Golf Course and new recycled water use areas with no or minimal booster pumping.

The rehabilitated Recycled Water Pumping Station will be designed to deliver up to 3,000¹⁷ gallons per minute (gpm) to the North Golf Course, new recycled water use areas, Lookout Hill Tank, and other future developments and the future Bass Lake Recycled Water Storage Tank. Each of the new pumps will be equipped with VFDs to minimize energy use and provide the ability to function efficiently under both operating scenarios (urban, residential and golf course irrigation).

Following rehabilitation, the Recycled Water Pumping Station will be used to transport recycled water from the equalization basin to the North Golf Course and to the following other recycled water use areas:

• Phase 1: District Office, Main Northgate, Stonehouse and Escuela Parks,

Murieta Gardens and The Retreats

• Buildout: Phase 1, Villages A, B and C, Residences of Murieta Hills, Apartments and

Industrial/Commercial/Residential

The pumping station will continue to have 3 vertical turbine pumps (2 duty, one standby). All 3 pumps will be equipped with VFDs to adjust pump speed. The pumping station will be designed to operate efficiently at anticipated modes of operation (i.e., Phase 1 and Buildout;). It has been assumed that the existing electrical service is sufficient to support the increased load, and that the existing motor control centers (MCCs) can house the MCCs for the new pumps. A new electrical service, upgrade or MCC building or structure is not anticipated to be required or included in the cost estimate.

^b Derived from calculations; actual supplementation requirements might vary depending on operations and when Phase 1 recycled water system is put into service

^c Value based on 8-hour urban irrigation demand

^d Golf course supply assumed to occur over 16 hour period between 6 am and 10 pm

 $^{^{17}}$ Equal to estimated maximum month / peak day urban recycled water demands. Modeling results indicate that lower capacity pumping station or recycled water storage tanks could be installed. System optimization was considered outside of the scope of work given the amount of work required to update the hydraulic model.

3.1.4: District Headquarters Connection Irrigation System

As shown in Figure 14, the two existing potable water irrigation services associated with the District's Administration Building will be disconnected at their Points of Service and connected to the Recycled Water Pumping Station for irrigation supply. Following modification, cross-connection testing will be conducted to verify that only the irrigation system is receiving recycled water and to ensure that potable water facilities are not connected to the recycled water system. As shown in Figure 14, 270 lineal ft of new 4-in PVC pipeline and associated appurtenances are anticipated to be required for this improvement.

3.2: Recommended Phase 1 Conveyance System Improvements

Recommended Phase 1 and Buildout Conveyance System Improvements are illustrated in Figure 15. Descriptions of the recommended Phase 1 Recycled Water Conveyance System Improvements are provided after Figure 16.

3.2.1: Northwest Recycled Water Transmission Main

The Northwest Recycled Water Transmission Main will convey recycled water from the Yellow Bridge (approximately) to Stonehouse and Escuela Parks and will be comprised of the following components (see Figure 15):

- a. **Highway 16 Undercrossing and Connection to Existing 12-inch ACP :** A new 12-inch pipeline and Highway 16 undercrossing are required to connect the recently installed 12-inch recycled water pipeline located along Legacy Lane within the Murieta Gardens development. Approximately length of this pipeline is 1,000 feet.
- b. 12-inch Legacy Lane Pipeline, Lookout Hill Storage Tank and Booster Pumping Station: The recently installed Legacy Lane pipeline will be extended northwest, towards Lookout Hill through the installation of a new 12-in pipeline which is proposed to follow Lone Pine Drive then up Lookout Hill to the existing tank site (along the existing roadway). This new pipeline (approximately 2,800 ft, PVC), in conjunction with other 12-inch pipelines shown in Figure 15 will be used to convey recycled water to the new Lookout Hill Tank shown in Figure 16. A new booster pumping station is needed to deliver recycled water to Stonehouse and Escuela Parks, the Main Northgate and in the future Residences of Murieta Hills from the tank. This new pumping station is proposed to be located near the base of Lookout Hill along Highway 16 near the District's Main Lift North and proposed to house two new booster pumps.
- c. Interconnecting Piping Between Booster Pump Station and Existing Force Main: A new transmission force main (approximately 2,400 ft, PVC) will be installed to connect the new Booster Pumping Station to the existing 12-inch sewer force main near the Main Lift North Station site. The proposed alignment of this new pipeline between Lone Pine Drive and the North Main Lift Station is between the hillside and the existing CIA Ditch.



Proposed North Main Gate Conversion



Proposed Stonehouse and Escuela Park Conversions



Proposed District Headquarters Conversion

Service Layer Credits: Source: Esri, DigitalGlobe, GeoEye, i-cubed, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AEX, Getmapping, Aerogrid, IGN, IGP, swisstopo, and the GIS User Community

Legend

- Recycled Water Transmission Main
- Proposed Recycled Water Service Line (4 inch)
- Proposed Recycled Water Irrigation Connection*

*Circle represents:



To be designed and constructed per RMCSD Recycled Water Standards

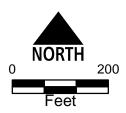


Figure 14: Irrigation System Conversions to Recycled Water

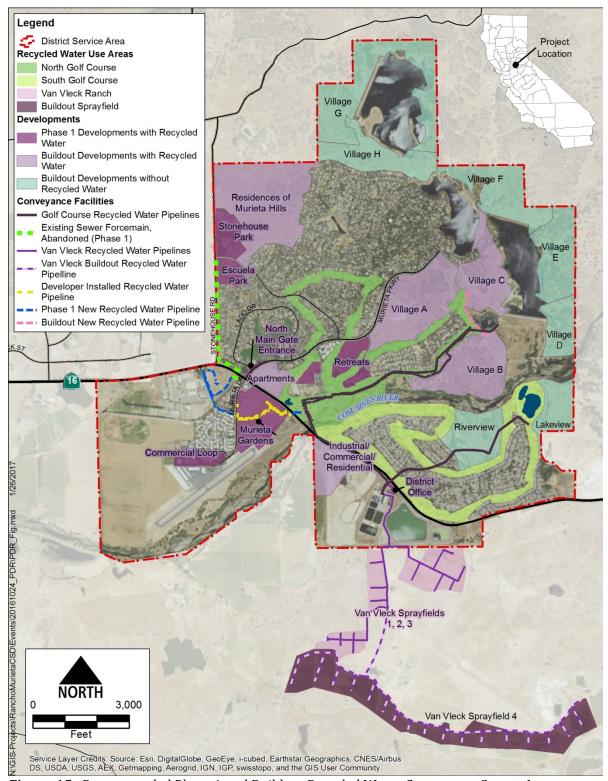
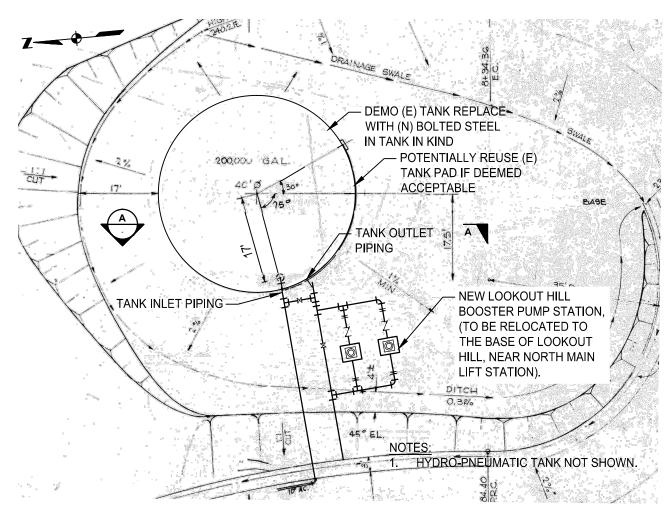
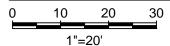
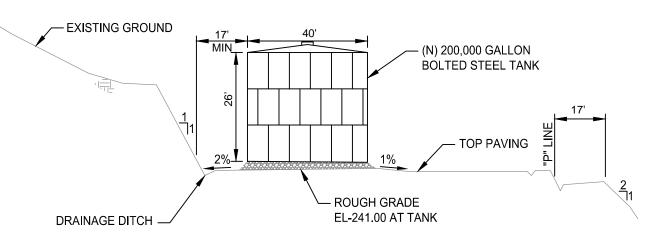


Figure 15. Recommended Phase 1 and Buildout Recycled Water Conveyance System Improvements



RECYCLED WATER STORAGE TANK SITE PLAN





SECTION A-A

HORIZ. 1"=20'
VERT. 1"=10'

Kennedy/Jenks Consultants

RANCHO MURIETA COMMUNITY SERVICE DISTRICT

LOOKOUT HILL STORAGE TANK

K/J 1670011*00 JAN 2017

Figure 16

d. **Existing 12-inch Force Main:** The existing 12-inch force main (5,400 ft abandoned sewer force main, not in use) that parallels Stonehouse Road and crosses under Highway 16 will be used for recycled water conveyance.

Considering both cost and constructability, a thin wall, non-structural liner solution in locations where the 12-inch sewer force main is deemed to be structurally viable is recommended. The renovation techniques anticipated and included within the cost estimate described in Section 4 are either a spray on cement lining, or a cure-in-place liner solution. Both techniques are similar in cost per linear foot and provide similar rehabilitation to the pipe.

Where a structural solution is deemed necessary, the recommended renovation/ replacement techniques are either open-cut pipe replacement or a pull in pipe-bursting solution (replacement). Open-cut pipe replacement is considered the more expensive of the two options in terms of cost per linear foot and is the technique assumed in the cost estimate.

3.2.2: Lookout Hill Water Storage Tank

Recycled water storage is required to supplement production capacities and satisfy peak irrigation demands. At this time, it is recommended that a total capacity of 200,000 gallons be provided to satisfy Phase 1 demands. System optimization should be performed using the updated hydraulic model (or something similar) to minimize cost of ownership during detailed design. Clear and specific objectives (e.g., reduce storage tank, operating and/or net present costs) and scenarios (e.g., Buildout, Phase 1, etc.) should be identified, defined and documented prior to initiating hydraulic modeling work.

The existing tank located near the top of Lookout Hill will be demolished and a new tank made of bolted panels with powder coated finish will be erected in its place or next to the existing tank. The external dimensions of this tank are approximately 40 foot diameter and 26 feet side wall height (see Figure 16). A booster pumping station will be located near the base of Lookout Hill to (1) provide adequate pressure to serve Stonehouse and Escuela Parks and Residences of Murieta Hills, in the future, and (2) maintain pressure above a minimum set point (e.g., 40 psi) when recycled water is only being supplied to the golf courses.

3.2.3: Escuela Park Conversion

The irrigation system for Escuela Park will be disconnected at the Point of Service and reconnected to the Northwest Recycled Water Transmission Main for recycled water irrigation supply (see Figure 14). It is assumed that the RMA, or other agency responsible for Escuela Park irrigation and management, will work with the District and submit an Application for Recycled Water Permit and Recycled Water Plan for review, consideration of approval and recycled water service in accordance with the District's Recycled Water Standards. As described in the District's Standards, the Recycled Water Plan shall describe how the proposed system is consistent with District Standards. It has also been assumed that RMA will relocate the Point of Service for recycled water irrigation to that shown in Figure 14 and make improvements necessary to improve their system and comply with recycled water requirements.

Cross connection testing is to be conducted prior to service to verify that only the irrigation system is receiving recycled water and to ensure that any potable water facilities within the proposed reuse area are not connected to the recycled water system. Costs for this conversion are based on installing a portion (up to 200 ft) of the new 4-in PVC pipeline shown in Figure 14 for Stonehouse

and Escuela Parks. It is assumed that this pipeline will be supplied by the common 4-inch pipeline located in Escuela Drive and described below in Stonehouse Park Conversion.

3.2.4: Stonehouse Park Conversion

The existing Stonehouse Park potable water irrigation service will be disconnected at the Point of Service and connected to the Northwest Recycled Water Transmission Main for recycled water irrigation supply (see Figure 14). It is assumed that the RMA, or other agency responsible for Stonehouse Park irrigation and management, will work with the District and submit an Application for Recycled Water Permit and Recycled Water Plan for review, approval and recycled water service in accordance with the District's Recycled Water Standards. As described in the District's Standards, the Recycled Water Plan shall describe how the proposed system is consistent with District Standards. Specific items of relevance to this proposed reuse area include protection of public health through (a) separate and continued potable water service to applicable buildings, structures, etc. (e.g., faucets, urinals, toilets, etc.) and (b) adequate setback for picnic tables, drinking fountains, etc. It has also been assumed that RMA will relocate the Point of Service for recycled water irrigation to that shown in Figure 14 and will make the improvements necessary to improve their system and comply with recycled water requirements.

Cross connection testing will also be required to verify that the irrigation system is only receiving recycled water and to ensure that the potable water system is not connected to the recycled water system. Approximately 275 ft of new 4-in PVC pipeline has been included in the cost estimate for this conversion. This pipe length assumes that the 4-inch recycled water pipeline is routed from Stonehouse Road along Escuela Drive and into Stonehouse Park as indicated in Figure 14.

3.2.5: Main Northgate Conversion

The existing irrigation system for the North Maingate will be disconnected from the potable water system and reconnected to the Northwest Recycled Water Transmission Main (see Figure 14). It is assumed that the RMA, or other agency responsible for irrigation and management at this particular location, will work with the District and submit an Application for Recycled Water Permit and Recycled Water Plan for review, approval and recycled water service in accordance with the District's Recycled Water Standards. As described in the District's Standards, the Recycled Water Plan shall describe how the proposed system is consistent with District Standards. Specific items of relevance to this proposed reuse area include protection of public health by (a) ensuring that storm drains, basins, etc. are located outside of the reuse area and (b) that overspray, runoff, etc. does not have the ability to enter surface water bodies. It has also been assumed that RMA will relocate the Point of Service for recycled water irrigation to that shown in Figure 14 and make other improvements, if necessary, to improve their system and comply with recycled water requirements.

Cross connection tests will be used to verify that only the irrigation system is receiving recycled water and to ensure that potable water facilities are not connected to the recycled water system. Up to 200 ft of new 4-in PVC pipeline and associated appurtenances has been allocated for this effort.

3.2.6: Murieta Gardens

Recycled water infrastructure and irrigations systems to serve the Murieta Gardens development is to be proposed by the developer and submitted to the District in a Recycled Water Plan for review and comment as described in the District's Recycled Water Standards (Section 1.3.4). Specific design requirements, components and elements will be identified as part of the Murieta Gardens Recycled Water Plan review and approval process and are not described in this PDR.

3.2.7: The Retreats

Recycled water infrastructure and irrigations systems to serve The Retreats development is to be proposed by the developer and submitted to the District in a Recycled Water Plan for review and comment as described in the District's Recycled Water Standards (Section 1.3.4). Specific design requirements, components and elements will be identified as part of The Retreats Recycled Water Plan review and approval process and are not described in this PDR.

3.3: Recommended Buildout Improvements

The following are descriptions of the recommended improvements to accommodate Buildout.

3.3.1: Disinfection Facilities Upgrade

Currently, the disinfection facilities have a rated capacity of 2.3 MGD and consist of an existing chlorine contact basin (CCB) and chlorine contact pipe (CCP). The CCP will be removed and an additional chlorine contact chamber will be added to increase disinfection facilities capacity from 2.3 to 3.0 MGD. The proposed chlorine contact chamber is shown in Figure 13.

As described in *WWRP Modified Chlorine Contact Disinfection System Compliance Report* (HSe, July 2006), the CCB was tested in 2003 for actual modal contact time at a flow of 1 and 3 MGD. The estimated modal contact time through the CCB at 3 MGD is 27 minutes. In accordance with Title 22, *disinfected tertiary recycled water* requires a minimum 90 minute modal contact time, therefore the proposed chlorine contact chamber is to have minimum modal contact time of 63 minutes.

A new concrete chlorine contact chamber is proposed to be installed next to the existing equalization basin at the WWRP to increase disinfection capacity. A 90 percent efficiency (e.g., baffling factor) was assumed for sizing of the new contact chamber. The new chlorine contact chamber will provide approximately 146,610 gallons for additional disinfection contact time and will consist of three passes following a serpentine configuration. The proposed chamber dimensions are 280 ft long, 7 ft wide and 10 ft deep,18 which equate to a length to width to depth ratio of 40:1:1.4, which is close to the target length to width to depth ratio of 40:1:1.5.

The water surface elevation of the new chlorine contact chamber will approximately match the elevation of the existing chlorine contact basin. The water surface elevation immediately downstream of the new chlorine contact chamber will approximately match the elevation of the existing equalization basin.

This improvement also includes the removal and disposal of the existing 20-inch CCP located inside the equalization basin.

Replacement of the third Tertiary Pump Station feed pump to the dissolved air flotation (DAF) units (\$100,000 allocation indicated in Table 11) is also required to increase WWRP production capacity from 2.3 to 3.0 MGD.

3.3.2: Existing North Golf Course Conveyance System Rehabilitation

The 12- and 8-inch conveyance pipelines that serves the North Golf Course represents the backbone of the existing recycled water system and are proposed to convey recycled water to additional reuse areas in the future (see Figure 15). Both ACP pipelines have been in service for over 30 years. It is necessary to conduct a condition assessment of these conveyance system assets to determine rehabilitation needs and ensure future performance and continued, uninterrupted service. Condition assessment is recommended to be conducted in two phases. Phase 1 would focus on the existing 12-inch ACP pipeline from WWRP to Yellow Bridge while Phase 2 focused on the existing 8-

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¹⁸ Dimensions do not include thickness of contact chamber walls.

inch ACP Pipeline to Bass Lake. Although these improvements have been designated as Buildout, the District should conduct assessments as soon as possible to better understand their condition and plan accordingly. Results specific to these pipelines will likely have ramifications to condition assessment recommendations proposed for the 12-inch sewer force main.

3.3.3: Bass Lake Recycled Water Storage Tanks:

Recycled water storage is required to supplement recycled water production capacities needed to satisfy projected Buildout peak irrigation demands. At this time, it has been recommended that a total capacity of 500,000 gallons be provided to satisfy Buildout demands.

3.3.4: Seasonal Storage Reservoir

A minimum of 40 AF of additional seasonal storage for secondary treated effluent is required to accommodate future development through Buildout. This addition could easily be met through expansion of the existing reservoir. Review of the existing ponds and levee system indicate the potential for cost effective expansion. Seasonal storage reservoir cost estimates presented in this PDR are based upon increasing the capacity of the existing storage reservoirs by 40 AF.

3.3.5: Van Vleck Sprayfield No. 4

Additional effluent disposal capacity will be required to accommodate above average levels of precipitation. As described in Table 11, additional recycled water transmission, distribution and irrigation system improvements are proposed into order increase sprayfield capacity on an additional 187 acres to accommodate wet weather scenarios for future growth.

3.3.6: Villages A, B, and C Developments

Recycled water infrastructure and irrigations systems to serve Villages A, B and C developments are to be proposed by the developers and submitted to the District in Recycled Water Plans for review and comment as described in the District's Recycled Water Standards (Section 1.3.4). Specific design requirements, components and elements will be identified as part of the review and approval process and are not described in this PDR.

Recycled Water Program Preliminary Design Report (Draft)

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Section 4: Project Implementation

This section presents the proposed construction sequencing and project scheduling. An estimate of probable construction costs is also included, along with a preliminary table of contents for the Phase 1 Recycled Water Improvements Project specifications and list of drawings.

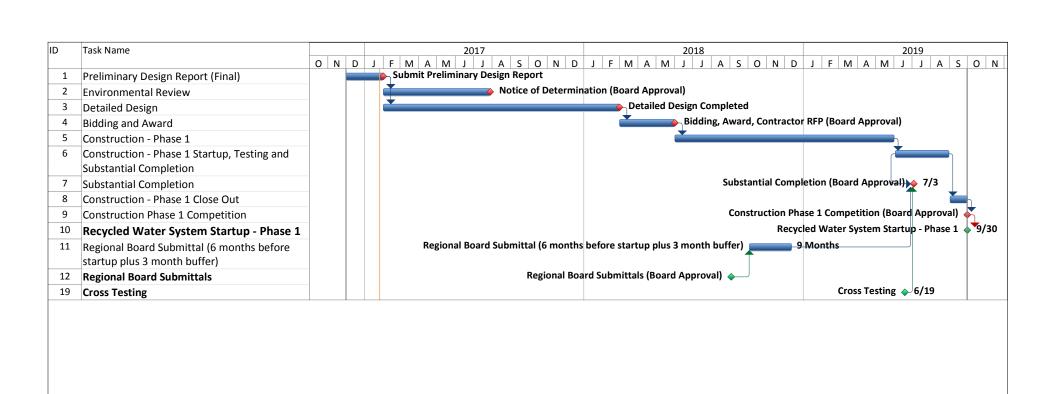
4.1: Construction Sequencing

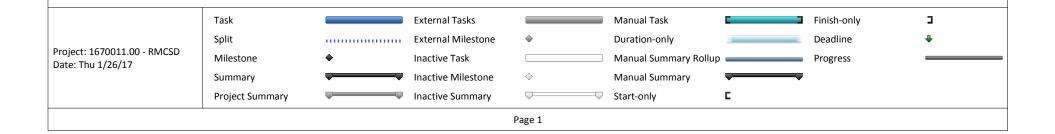
The sequence of construction for the majority of the Phase 1 Recycled Water Improvements Project is expected to be relatively straightforward provided that the following tie-ins / connections into existing recycled water infrastructure are conducted during the wet season, when recycled water production and conveyance system are not in operation (typically between October 15 through April). If designed, planned and coordinated properly, each of these tie-ins are expected to be relatively short in duration and can be scheduled during the wet season.

- WWRP Improvements (Wet Season Tie-Ins and Critical Activities)
 - o Recycled Water Pumping Station
 - Rehabilitation.
 - Tie into existing Equalization Basin at WWRP.
 - Tie into <u>existing</u> 12-inch ACP North Golf Course Conveyance pipeline at WWRP.
 - Tie in (2) into <u>existing</u> District Headquarters irrigation system and conduct cross-connection testing.
- Northwest Recycled Water Transmission Main (Wet Season Tie Ins and Critical Activities)
 - New Highway 16 undercrossing pipeline tie ins (2) to <u>existing</u> 12-inch ACP North Golf Course Pipeline and recently installed 12-inch Legacy Lane pipeline.
 - New 12-inch Lone Pine Drive / Murieta Drive pipeline tie in to recently installed
 12-inch Legacy Lane pipeline.
 - New 12-inch Lone Pine Drive / Murieta Drive pipeline tie in to new Lookout Hill Recycled Water Storage Tank.
 - New 12-inch Lone Pine Drive / Murieta Drive pipeline tie in to new Recycled Water Booster Pump Station.
 - New 12-inch recycled water pipeline tie in to abandoned 12-inch Force Main.
 - Abandoned 12-inch Force Main tie ins (3) to <u>existing</u> Escuela and Stonehouse Park and Main North Gate Entrance irrigation systems.
- Reuse Areas Conversions
 - o Existing Main Northgate Irrigation System Modifications
 - o <u>Existing District Headquarters Irrigation System Modifications</u>
 - o Existing Escuela Park Irrigation System Modifications
 - o <u>Existing Stonehouse Park Irrigation System Modifications</u>

4.2: Project Implementation Schedule

A project implementation schedule for Phase 1 Recycled Water Improvements Project is presented in Figure 17. The proposed schedule is based on anticipated timelines for completion of major tasks and activities required for implementation and <u>not</u> on meeting a specific timeline or deadline. The implementation schedule indicates that the Phase 1 recycled water system could be initiated for service mid-2019 and that the Phase 1 improvements are estimate to require about 30 months to complete once this PDR has been finalized. This timeline, which should be verified with an environmental consultant, assumes a maximum 6-month timeline for environment consultation and review.





Buildout improvements are anticipated to require approximately 3 years for completion of all major activities such as preliminary design, environmental review, detailed design, construction, startup and testing and close out. Similar to what is illustrated in Figure 17, it is recommended that future Buildout reuse areas obtain District approval no less than 12 months before system startup. Cross connection testing should be conducted just before startup of the Buildout system startup.

The rated ADWF capacity of the existing seasonal storage reservoirs has been established at 0.65 MGD in the WDR. Review of Figure 6 indicates that the ADWF is projected to approach 0.65 MGD around 2023. The District should initiate the expansion of the seasonal storage reservoir no later than January 2020 based on this development schedule. A construction sequencing plan should be established early in the project to determine the best and most cost effective means for increasing the height of the existing secondary storage reservoir berms while maintaining the District's ability to continuously operate and store secondary effluent.

4.3: Construction Documents

A preliminary list of drawings is shown in Table 13 following by a preliminary list of specifications in Table 14. for the Phase 1 Recycled Water Improvements Project Improvements.

Table 13. Preliminary List of Drawings – Phase 1 Recycled Water

Drawing		J Druwings – Thuse T Necycleu Water	
No.	Discipline	Drawing Title	
1	General	Title Sheet, Vicinity Map and Drawing List	
2		General Notes and Abbreviations	
3		Mechanical Legend, Schedules and Notes	
4		Electrical Legend, Schedules and Notes	
	1	Recycled Water SCADA Control System	
5		P&ID 1	
6		P&ID 2	
7		P&ID 3	
8		PLC	
	2	Equalization Basin Potable Water Air Gap Connection	
9		Civil Plan and Profile	
10		Civil Detail	
	3	Recycled Water Pump Station	
11		Civil - Site Plan	
12		Civil Discharge Piping	
13		Mechanical - Recycled Water Booster Pump Station	
14		Mechanical - Details	
15		Electrical - Power, Control, and Instrumentation	
		District Headquarters Conversion - Recycled Water Irrigation System	
1.6	4	Connection	
16		Civil - Site Plan	
17	_	Civil - Details	
10	5	Northwest Recycled Water Transmission Main	
18		Civil - Plan and Profile 1	
19		Civil - Plan and Profile 2	
20		Civil - Plan and Profile 3	

Dissiplins	Drawing Title	
Discipline	Drawing Title	
+	Civil - Plan and Profile 4	
_	Civil - Plan and Profile 5	
	Civil - Plan and Profile 6	
	Civil - Plan and Profile 7	
	Civil - Plan and Profile 8	
	Civil - Plan and Profile 9	
	Civil - Plan and Profile 10	
	Civil - Plan and Profile 11	
	Civil - Plan and Profile 12	
	Civil - Plan and Profile 13	
	Civil - Plan and Profile 14	
	Civil - Details 1	
	Civil - Details 2	
	Civil - Details 3	
6	Recycled Water Booster Pumping Station	
	Civil - Site Plan	
	Civil Discharge Piping	
	Mechanical - Lookout Hill Booster Pump Station	
	Mechanical - Details	
	Electrical - Power, Control, and Instrumentation	
7	Escuela Park Conversion - Recycled Water Irrigation System Connection	
	Civil - Site Plan	
	Civil - Details	
8	Stonehouse Park Conversion - Recycled Water Irrigation System Connection	
	Civil - Site Plan	
	Civil - Details	
9	Lookout Hill Recycled Water Storage Tank	
	Civil - Site Piping Detail Plan	
	Civil - Storage Tank Plan and Section	
	Civil - Storage Tank Details 1	
	Civil - Storage Tank Details 2	
	Mechanical - Storage Tank Details 1	
	Mechanical - Storage Tank Details 2	
10	Main North Gain Entrance Conversion - Recycled Water Irrigation System Connection	
	Civil - Site Plan	
	Civil - Details	
	8	

Table 14. Preliminary List of Specifications – Phase 1 Recycled Water Improvements

	reliminary List of Specifications – Phase 1 Recycled Water Improvements	
Spec. No.	Description	
Bidding Requ	nirements	
00010	Invitation to Bid	
00100	Instructions to Bidders	
00200	Information Available to Bidders	
00300	Bid Form	
00410	Bid Security	
00414	Security for Compensation Certificate – California Requirement	
00416	Bidder's References	
00420	Bidder's Qualifications	
00430	Subcontractor List	
00480	Noncollusion Affidavit - California Requirement	
Contract For	ms	
00500	Agreement	
00610	Performance Bond – California Version	
00620	Payment Bond - California Version	
Contract Con		
00700	General Conditions – Pre-defined Standard	
00800	Supplementary Conditions – California Version	
	General Requirements	
01010	Summary of the Work and Contract Considerations	
01040	Coordination and Project Requirements	
01140CA3	Environmental Protection	
001300	Submittals	
01500	Construction Facilities and Temporary Controls	
01550	Traffic Regulation	
01650	Facility Startup	
01700	Contract Closeout	
Division 2 - 9	Site Work	
02050	Demolition	
02200	Site Preparation	
02302	Earthwork – For Pipelines	
02370	Slope Protection	
02700	Paving and Surfacing	
02775	Concrete Curb, Gutters and Sidewalks	
02820	Fences and Gates	
02905	Landscape Planting and Irrigation	
Division 3 - 0		
03200	Reinforcing Steel	
03300	Cast-In-Place Concrete	
Division 5 – I	Metals	
05722	Aluminum Handrails, Guardrails and Related Items	
Division 9 - I	_	
09900	Painting	
09960	High Performance Coatings	
09960A	Appendix A: Standards and References and Mandatory Quality Control Testing	
009960B	Appendix B: Coating Detail Sheets, High Performance Coatings	
Division 11 -	Equipment	
11215	Vertical Turbine Pumps	
Division 13 -	Special Construction	
13212	Bolted Steel Tank	
•		

Spec. No.	Description
Division 15 -	•
15050	Piping, Valves and Accessories
Division 16 -	-
16000	Electrical Work
16010	General Electrical Requirements
16110	Conduit, Raceways and Fittings
16120	Low Voltage Wire and Cable
16122	Medium Voltage Cable
16124	Signal Cable
16130	Boxes
16140	Wiring Devices
16155	Motor Starters
16160	Panelboards
16165	Load Centers
16180	Protective Devices and Switches
16205	Standby Diesel Engine-Generator Sets
16250	Automatic and Non-Automatic Transfer Switches
16325	Step Voltage Regulator
16330	Capacitor Switchgear
16401	Overhead Electrical Work
16402	Underground Electrical Service System
16405	Switchboards
16406	Medium Voltage Switchgear
16450	Electrical Grounding
16520	Exterior Lighting
16611	Uninterruptible Power Supply (UPS)
16613	Regulated Power Supplies
16615	Power Distribution Units
16760	Plant Communications Systems
16762	Telephone and Paging Systems
16800	Modifications to Existing Facilities
16890	Electric Heaters
16920	Motor Control Center(s)
16923	Slip Energy Recovery Drives (SER)
16929	Medium Voltage Motor Starter(s)
16930	Power Factor Control Equipment
16945	Control Remote Control Relays
16955	Control Devices
16999	Intrinsically Safe Systems
	Instrumentation and Controls
17010	Instrumentation and Controls, General Requirements
17010.1	Figure 1 - Loop Diagram
17010.2	Figures 2 (Interconnection Diagram), 3 (Elementary Diagram), and 4 (Equipment Wiring Diagrams)
17015	Operational Availability Demonstration
17018	Performance (Availability) Warranty
17110	Analytical Instruments
17110	Flow Measurement
17140	Level Measurement
17150	Pressure Measurement
17200	Panel Mounted and Miscellaneous Field Instruments
17320	Process Control System
1/340	1100000 0011101 03300111

Spec. No.	Description
17321	Microcomputer Based SCADA System
17330	Programmable Logic Controller
17330.1	Appendix - PLC Process Control Strategies
17335	Process Control Unit
17340	Data Acquisition and Logging System
17341	Data Acquisition and Logging System - Microcomputer Type
17421	Tone Telemetry System
17423	Remote Telemetry Units
17425	Radio Telemetry System
17430	Intelligent Multiplexing System
17510	Panels

4.4: Estimate of Probable Construction Cost

The estimated probable construction and project costs for the recommended Phase 1 improvements are \$4,060,000 and \$5,380,000, respectively as shown in Table 15. A detailed breakdown of this cost estimate is included in the Appendix.

Phase 1 costs exceed the District's budget discussed at the December 16, 2016 Board meeting (i.e., \$4 million). Potential ways to reduce Phase 1 costs and deliver the project within budget include:

 Postpone 12-inch sewer force main improvements (potential impact could be a delay in providing recycled water to Stonehouse and Escuela Parks, and Residences of Murieta Hills)

As shown at the bottom of Table 15, Recycled Water Program costs are estimated to be about \$6,055 per equivalent residential home. The following is a listing of current connection fees for other nearby and/or similar agencies for comparison purposes:

• Sacramento Regional CSD: \$3,358 infill; \$5,523 new areas

• City of Roseville: \$7,802

• Calaveras County Water District: \$5,500-\$17,293 depending on service area

Table 15. Recommended Recycled Water Improvements and Estimated Costs

No.	Improvement	Estimated Cost (\$) ^a				
1101	Phase 1 Recycled Water Improvements					
1	Recycled Water SCADA Control System	250,000				
2	Equalization Basin Potable Water Air Gap	76,000				
3	Recycled Water Pumping Station	1,045,000				
4	District Headquarters Conversion	20,000				
5	Northwest Recycled Water Transmission Main	1,441,000				
6	Lookout Hill Booster Pumping Station	612,000				
7	Escuela Park Conversion	16,000				
8	Stonehouse Park Conversion	36,000				
9	Lookout Hill Recycled Water Storage Tank	545,000				
10	Main NorthgateConversion	18,000				
11	Commercial Loop Conversion	TBD				
	Phase 1 Subtotal (Estimated Construction Cost)	4,060,000				
12	Soft Costs – 32.5% (Admin., Reg., Eng., Construct Man.)	1,319,500				
	Phase 1 Total (Project Cost)	5,380,000b				
	Buildout Recycled Water Improvement	ents				
13	SCADA Upgrades	82,000				
14	Disinfection Facilities Upgrade	665,000				
15	North Golf Course Conveyance System	1,620,000				
16	Bass Lake Tank	1,216,000				
17	Bass Lake Booster Pumping Station	625,000				
18	Seasonal Storage Reservoir Expansion	839,000				
19	Van Vleck Sprayfield 4	890,000				
20	DAF Pumping Replacement	100,000				
	Buildout Subtotal (Estimated Construction Cost)	6,030,000 ^c				
21	Soft Costs – 32.5% (Admin., Reg., Eng., Construct Man.)	1,960,000				
	Buildout Total (Project Cost)	7,990,000				
Phase 1 and Buildout Recycled Water Improvements						
	Grand Total (Phase 1 and Buildout)	13,400,000 ^d				
	Estimated Number of New Equivalent Residential Units	2,213				
	Estimated Cost per Connection (\$/ERU)	\$6,055				

^a Estimated costs based upon Engineering News Record (ENR) 20 City Average Construction Cost Index (CCI) at 10,385 (August 2016)

^b Compared to \$10,014,000 (\$9,100,000 adjusted for inflation) as described previously in the District's Title XVI Recycled Water Feasibility Study

^c Compared to \$15,055,000 as described previously in the District's Title XVI Recycled Water Feasibility Study

d Compared to \$25,070,000 as described previously in the District's Title XVI Recycled Water Feasibility Study

<u>Appendix</u>			

KENNEDY/JENKS CONSULTANTS

OPINION OF PROBABLE CONSTRUCTION COST

BASIS OF ESTIMATE

PROJECT INFORMATION

Client: Rancho Murrieta

Project: Recycled Water System

 KJ Job No.:
 1670011*00

 Estimate Date:
 12/2/2016

 Prepared By:
 JLH

 Reviewed By:
 KAK

Estimate Type: Preliminary **AACEI Estimate Classification** Class 4

AAOLI LSIIIIate Olassiileatio

PROJECT DESCRIPTION:

The scope of work for this project includes: Recycled Water System components including water storage tanks, pump stations, new recycled water conveyance, connections to convert existing irrigation systems to recycled water use, and control features as described in the report.

ESTIMATE DOCUMENTS:

DRAWINGS: N/A

DOCUMENTS: Predesign Report & Figures

SOURCE OF COST DATA:

Published cost estimating data, engineers experience on similar projects.

ESTIMATE ASSUMPTIONS:

The followings assumptions were made in the preparation of this estimate:

Project will be publicly bid project.

Native backfill will be suitable for use in utility trenches.

No signficant dewatering of groundwater in excavation will be required.

Additional detail of assumed items is included in detailed estimate breakdown.

SPECIFIC INCLUSIONS:

Soft costs have been included with the following percentages allocations: Administration (5%), Regulatory/ CEQA Compliance(2.5%), Engineering & Construction Management (15%), Soft Cost Contingency (10%)

SPECIFIC EXCLUSIONS:

The estimate does not include the following: Asbestos / Lead abatement. Hazardous or Special Waste removal or disposal Soil remediation

MAJOR CHANGES FROM PREVIOUS ESTIMATE:

DESIGN CONTINGENCY:

A design contingency of 30 % has been included.

Note: This allowance is intended to provide a Design Contingency allowance. It is not intended to provide for a Construction Contingency for change orders during construction or to cover unforeseen conditions.

ESCALATION:

An escalation factor has not been included. The owner is cautioned that the project cost should be adjusted for the project schedule.

Current ENR CCI	Aug-16	10385	
Annual Inflation Escalation Factor:		3.0%	
Time Until Project Midpoint (Months)			Number of months

ACCURACY:

The level of accuracy is commensurate with levels developed by the AACEI, the Association for the Advancement of Cost Engineering International. At increasing levels of design completion, the narrower the range between upper and lower limits and the greater the accuracy of the estimate. This estimate is considered a Class 4 level estimate in accordance with AACEI guidelines. Typically this level of estimate has an expected accuracy range of +50%, -30%. This estimate is based upon competitive bidding, which assumes receipt of multiple bids from five or more General Contractors. Without competitive bidding, pricing can vary significantly from the prices assumed in this estimate.

The enclosed Engineer's Estimate of Probable Construction Cost is only an opinion of possible items that maybe considered for budgeting purposes. This Project Estimate is limited to the conditions existing at issuance and is not a guaranty of actual construction cost or schedule. Uncertain market conditions such as, but not limited to, local labor or contractor availability, wages, other work, material market fluctuations, price escalations, force majeure events and developing bidding conditions, etc. may affect the accuracy of this review. Kennedy/Jenks is not responsible for any variance from this Project Estimate or actual prices and conditions obtained.

OTHER COMMENTS:

KENNEDY/JENKS CONSULTANTS

Project: Rancho Murrieta Prepared By:

Date Prepared: 25-Jan-17 K/J Proj. No.: **Building, Area:** 1670011*00 Recycled Water

Estimate Type: Preliminary

	SUMMARY BY AREA	·
ITEM NO.	ITEM DESCRIPTION	TOTAL
Phase 1		
1	Recycled Water SCADA Control System	250,000
2	Equalization Basin Potable Water Air Gap connection	76,000
3	Recycled Water Pumping Station	1,045,000
4	District Headquarters Conversion Irrigation Connection	20,000
5	NW Recycled Water Transmission Main	1,441,000
6	Lookout Hill Booster Pumping Station	612,000
7	Escuela Park Conversion - Recycled Water Irrigation Connection	16,000
8	Stonehouse Park Conversion - Recycled Water Irrigation Connection	36,000
9	Lookout Hill Water StorageTank	545,000
10	North Main Gate Conversion - Recycled Water Irrigation Connection	18,000
	Phase 1 Subtotal	4,060,000
	Soft Costs (Admin, Regulatory, Engineering, CM, Contingency) 33%	1,319,500
	Phase 1 Subtotal	5,380,000
Build out		
	OOADA Oostaal Oostaaa Baaa Lalaa Taalii Kaasa	00.000
1B	SCADA Control System Bass Lake Tank Items	82,000
11	Disinfection Facilties Upgrade	665,000
12 13	North Golf Course Conveyance System Rehabilitation	1,620,000
	Bass Lake Recyled Water Storage Tank	1,216,000
14	Bass Lake Booster Pump Station	625,000
15	Seasonal Storage Reservior	831,000

ITEM NO.	ITEM DESCRIPTION	TOTAL	
16	Van Vleck Sprayfield 4		890,000
17	DAF Pump Replacement		100,000
	Buildout Subtotal		6,030,000
	Soft Costs (Admin, Regulatory, Engineering, CM, Contingency)	33%	1,959,750
	Phase 1 Subtotal		7,990,000
	TOTAL		13,400,000

Estimate Accuracy					
+50%	-30%				

ſ	50%	Total Est.	-30%
Ī	\$20,100,000	\$13,400,000	\$9,380,000

Project:	Rancho	Murrieta					Prepared By:	
Building,	Area:	Recycled Water SCADA Control System					Date Prepared: K/J Proj. No.	
Estimate Type Conceptual Preliminary (w/o plans)				Construc Change (Order	Mor	Current at ENR Escalated to ENR hths to Midpoint of Construct	
Spec.	Item	Design Development @			Materials	Installation	Sub-contractor	<u> </u>
No.	No.	Description	Qty	Units	\$/Unit Total	\$/Unit Total	\$/Unit Total	Total

Spec.	Item	<u>† </u>			Materials		Instal	lation	Sub-contractor		
No.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
Phase 1											
		PLC System at Lookout Hill Booster Pump Station	1	EA							
		RTU/ Wireless I/O	2	LOC					25,000	50,000	50,000
		Control Valves and Control System Elements:							·	·	·
		Recycled Water Pump Station Pressure Reducing Valve 12"	1	EA	12,000.00	12,000	500.00	500			12,500
		Recycled Water Pump Station Flow Meter 12"	1	EA	8,000.00	8,000	4,500.00	4,500			12,500
		Recycled Water Pump Station Pressure Transmitter	1	EA	3,500.00	3,500	4,500.00	4,500			8,000
		Lookout Hill Flow Control Valve 12" Actuated Valve	1	EA	4,500.00	4,500	4,500.00	4,500			9,000
		Lookout Hill Tank Altitude Valve 12"	1	EA	13,400.00	13,400	500.00	500			13,900
		Lookout Hill Booster Pump Station Pressure Transmitter	1	EA	3,500.00	3,500	4,500.00	4,500			8,000
		Power Drop / Meter at Actuated Valve at Branch	1	EA					5,000	5,000	5,000
		Power to Above Items	6	EA					5,000	30,000	30,000
		Subtotals				44,900		19,000		85,000	148,900
		Division 1 Costs	@	10%		4,490		1,900		8,500	14,890
		Subtotals				49,390		20,900		93,500	163,790
		Taxes - Materials Costs	@	8.75%		4,322					4,322
		Subtotals				53,712		20,900		93,500	168,112
		Taxes - Labor Costs	@	5.00%				1,045			1,045
		Subtotals				53,712		21,945		93,500	169,157
		Contractor Markup for Sub	@	12%						11,220	11,220
		Subtotals				53,712		21,945		104,720	180,377
		Contractor OH&P	@	15%		8,057		3,292			11,348
		Subtotals				61,768		25,237		104,720	191,725
		Estimate Contingency	@	30%							57,518
		Subtotals									249,243
		Escalate to Midpoint of Construct (per year)	@	3%							
		Estimated Bid Cost									249,243
		Total Estimate									250,000

Estimate	Accuracy
+50%	-30%

Estimated	Estimated Range of Probable Cost										
+50%	Total Est.	-30%									
\$375,000	\$250,000	\$175,000									

Project:	Ranch	o Murrieta								Prepared By:	
									Da	ate Prepared:	JLH
Building, Ar	ea:	Equalization Basin Potable Water Air	Gap connection							K/J Proj. No.	1670011*00
									Cı	urrent at ENR	
Estimate Ty	pe:	Conceptual		Constru	ction				Esca	alated to ENR	
		Preliminary (w/o plans)		Change Order				Months to Midpoint of Construct			
_		Design Development @		์ % Comp	lete						
Spec.	Item				Mate		Instal			ontractor	
Spec.	Item	Description	Othe	Linita	Mate	ials Total	Install	ation	Sub-c	ontractor	Total

Spec.	Item				Materials			lation	Sub-contractor		
No.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
				 							
		Tapped Connection to Existing Pipe	11	EA	1,475.00	1,475	510.00	510			1,985
		8" DI Pipe incl Trenching	20	LF	34.50	690	30.00	600			1,290
		8" FCA	2	EA	500.00	1,000	200.00	400			1,400
		8" Fittings	4	EA	450.00	1,800	150.00	600			2,400
		8" Butterfly Valve	2	EA	1,000.00	2,000	250.00	500			2,500
		8" Flow Meter	1	EA	6,000.00	6,000	800.00	800			6,800
		8" Actuated Valve	1	EA	5,000.00	5,000	500.00	500			5,500
		Paving Restoration	13	SY					75	1,000	1,000
		Electrical for Meter/ Valve	1	LS					15,000	15,000	15,000
		Underground Electrical Conduit	200	LF					35	7,000	7,000
		Subtotals				17,965		3,910		23,000	44,875
		Division 1 Costs	@	10%		1,797		391		2,300	4,488
		Subtotals				19,762		4,301		25,300	49,363
		Taxes - Materials Costs	@	8.75%		1,729					1,729
		Subtotals				21,491		4,301		25,300	51,092
		Taxes - Labor Costs	@	5.00%				215			215
		Subtotals				21,491		4,516		25,300	51,307
		Contractor Markup for Sub	@	12%		ŕ		í		3,036	3,036
		Subtotals				21,491		4,516		28,336	54,343
		Contractor OH&P	@	15%		3,224		677		,	3,901
		Subtotals				24,714		5,193		28,336	58,244
		Estimate Contingency	@	30%		ŕ		,		,	17,473
		Subtotals									75,717
		Escalate to Midpoint of Construct	@	3%							-
		Estimated Bid Cost									75,717
		Total Estimate									76,000

Estimate Accuracy					
+50%	-30%				

Estimated Range of Probable Cost										
+50%	Total Est.	-30%								
\$114,000	\$76,000	\$53,200								

KENNEDY/JENKS CONSULTANTS

Project:	Rancho	Murrieta		Prepared By:	
				Date Prepared:	JLH
Building, A	ea:	Recycled Water Pumping Station		K/J Proj. No.	1670011*00
				Current at ENR	
Estimate Ty	pe:	Conceptual	Construction	Escalated to ENR	
		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct _	
		Design Development @	% Complete		

		Design Development @		% Complete							
Spec.	Item					erials		lation		ontractor	
No.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
	+	Modification to Existing Pump Station Structure	1	LS			50,000.00	50,000		+	50,000
		Generator Slab	11	CY	250.00	2,667	250.00	2,667			5,333
		Centrator Clab	''	01	200.00	2,007	200.00	2,007			0,000
		Vertical Turbine Pumps	3	EA	47,200.00	141,600	10,000.00	30,000			171,600
		Pump Discharge Piping:									
		10" Fittings/ Spools	12	EA	500.00	6,000	200.00	2,400			8,400
		10" Flex Connector	3	EA	800.00	2,400	250.00	750			3,150
		10" Check Valve	3	EA	3,700.00	11,100	250.00	750			11,850
		10" Butterfly Valve	3	EA	1,200.00	3,600	200.00	600			4,200
		10" FCA	3	EA	800.00	2,400	250.00	750			3,150
		Pipe Supports	6	EA	150.00	900	100.00	600			1,500
		CARV	3	EA	400.00	1,200	200.00	600			1,800
		Tee	3	EA	800.00	2,400	350.00	1,050			3,450
		12" Discharge Header	40	LF	60.00	2,400	25.00	1,000			3,400
		Pressure Gage	3	EA	250.00	750	150.00	450			1,200
		Electrical / I&C for Pumps (from Existing MCC's)	1	LS					180,000	180,000	180,000
		VFD's 250HP (in Existing MCCs)	3	EA	26,000.00	78,000	3,000.00	9,000	100,000	100,000	87,000
		Level Transitter	1	EA	4,000.00	4,000	2,500.00	2,500			6,500
	1	Emergency Generator 250KW w/ ATS & Fuel Tank	1	EA	53,500.00	53,500	11,000.00	11,000		1	64,500
					,	·	,	,			,
		Subtotals				312,917		114,117		180,000	607,033
		Division 1 Costs	@	10%		31,292		11,412		18,000	60,703
		Subtotals				344,208		125,528		198,000	667,737
		Taxes - Materials Costs	@	8.75%		30,118					30,118
		Subtotals				374,327		125,528		198,000	697,855
		Taxes - Labor Costs	@	5.00%				6,276			6,276
		Subtotals				374,327		131,805		198,000	704,131
		Contractor Markup for Sub	@	12%						23,760	23,760
		Subtotals				374,327		131,805		221,760	727,891
		Contractor OH&P	@	15%		56,149		19,771			75,920
		Subtotals				430,476		151,575		221,760	803,811
		Estimate Contingency	@	30%							241,143
		Subtotals									1,044,954
		Escalate to Midpoint of Construct	@	3%							
		Estimated Bid Cost									1044954
		Total Estimate									1,045,000

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	+50%	-30%
•		
Fetimate	d Range of Pr	ohahle Cost

Estimate Accuracy

Estimate	ed Range of Pi	Estimated Range of Probable Cost										
+50%	Total Est.	-30%										
\$1,567,500	\$1,045,000	\$731,500										

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Project:	Rancho N	Murrieta		_ Prepared By: Date Prepared: JL							
Building, Are	a:	District Headquarters Conversion I	rrigation Co			K/J Proj. No.					
Estimate Typ	e:	Conceptual Preliminary (w/o plans) Design Development @		Construction Change % Comp	Order			Monti	Esca	urrent at ENR alated to ENR t of Construct	
Spec. No.	Item No.	Description	Qty	Units	Mate \$/Unit	erials Total	Instal \$/Unit	lation Total	Sub-c \$/Unit	ontractor Total	Total
		Connection Piping appurtenances	2	LS	500.00	1,000	500.00	1,000			2,000
		4" PVC Pipeline	270	LF	8.00	2,160	17.00	4,590			6,750
		Lanscaping Restoration	180	SY					10	1,800	1,800

	item				iviate	eriais	เมอเลเ	iation	Sub-c	ontractor	
Йо.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Connection Piping appurtenances	2	LS	500.00	1,000	500.00	1,000			2,000
		4" PVC Pipeline	270	LF	8.00	2,160	17.00	4,590			6,750
		Lanscaping Restoration	180	SY					10	1,800	1,800
		Cross Connection Testing	1	LS			1,000.00	1,000			1,000
		Subtotals		1		3,160		6,590		1,800	11,550
		Division 1 Costs	@	10%		316		659		180	1,155
		Subtotals				3,476		7,249		1,980	12,705
		Taxes - Materials Costs	@	8.75%		304					304
		Subtotals				3,780		7,249		1,980	13,009
		Taxes - Labor Costs	@	5.00%				362			362
		Subtotals				3,780		7,611		1,980	13,372
		Contractor Markup for Sub	@	12%						238	238
		Subtotals				3,780		7,611		2,218	13,609
		Contractor OH&P	@	15%		567		1,142			1,709
		Subtotals				4,347		8,753		2,218	15,318
		Estimate Contingency	@	30%							4,595
		Subtotals									19,913
		Escalate to Midpoint of Construct	@	3%							
		Estimated Bid Cost									19,913
		Total Estimate									20,000

Estimate	Accuracy
+50%	-30%

Estimated Range of Probable Cost										
+50%	Total Est.	-30%								
\$30,000	\$20,000	\$14,000								

KENNEDY/JENKS CONSULTANTS

Project:	Rancho	o Murrieta		Prepared By:	
-				Date Prepared:	JLH
Building, A	rea:	NW Recycled Water Transmissi	on Main	K/J Proj. No.	1670011*00
				Current at ENR	
Estimate Ty	ype:	Conceptual	Construction	Escalated to ENR	
		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct	
		Design Development @	% Complete		

		Design Development @		. % Comp	lete						
Spec.	Item		Materials Installation		lation	Sub-	contractor				
Ñо.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
Highway 16 Und	dercrossing a	and Connection to Existing 12"ACP									
		Connection to Existing	1	EA	500.00	500	500.00	500			1,000
		12" PVC Pipeline	1,000	LF	23.50	23,500	57.50	57,500			81,000
		12" Fittings Rest Jnt	8	EA	635.00	5,292	125.00	1,042			6,333
		AAV Assembly	1	EA	2,500.00	2,500	500.00	500			3,000
		Paving Removal (legacy lane/ Lon		SY					10		
		Paving Restoration		SY					75		
		Traffic Control		DY	250.00		1,040.00				
latana ana atian		J			l and Dina Drive					_	
interconnecting	piping betw	een Legacy Lane & Lookout Hill Storage Ta	ank (Along Le			500	500.00	500			1.000
		Connection to Existing	7 500	EA LF	500.00 23.50	500	500.00	500			1,000
		12" PVC Pipeline (along Legacy &	2,500			58,750	26.00	65,000		_	123,750
		12" PVC Pipeline (up hill)	300	LF	23.50	7,050	26.00	7,800		_	14,850
		12" Fittings Rest Jnt	21	EA	635.00	13,229	125.00	2,604			15,833
		AAV Assembly	1	EA	2,500.00	2,500	500.00	500	40	10.007	3,000
		Paving Removal (legacy lane/ Lon	1,667	SY					10	16,667	16,667
		Paving Restoration	1,667	SY			4 0 40 00		75	125,000	125,000
		Traffic Control	25	DY	250.00	6,250	1,040.00	26,000			32,250
Lookout Hill Boo	oster pump	L Station to Existing FM Connection (down hill	, along Lone p	ine drive, th	rough CIA ditch)						
	T	Connection at Pump Station	1	EA	500.00	500	500.00	500			1,000
		12" PVC Pipeline	1,550	LF	23.50	36,425	26.00	40,300			76,725
	+	12" PVC Pipeline (along cia ditch)	850	LF	23.50	19,975	26.00	22,100			42,075
		12" Fittings	20	EA	635.00	12,700	125.00	2,500			15,200
		AAV Assembly	1	EA	2,500.00	2,500	500.00	500			3,000
		Connection to Existing FM	1	EA	500.00	500	500.00	500			1,000
		Ditch Restoration	567	SY			5.00	2,833			2,833
	1	Traffic Control	16	DY	250.00	3,875	1,040.00	16,120			19,995
Existing 12" For	rcemain Reh	nabilitation (along Stonehouse Road)									
		Pipeline Assesment	5,400	LF					10	54,000	54,000
		Pipeline Repair - CIPP (66%)	2,400	LF					59	142,200	142,200
		12" PVC Pipeline (33% replaced)	1,200	LF	23.50	28,200	26.00	31,200			59,400
		12" Pipe Removal	1,200	LF			8.00	9,600			9,600

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Traffic Control		DY	250.00		1,040.00		
Subtotals				224,746	288,099	337,867	850,712
Division 1 Costs	@	10%		22,475	28,810	33,787	85,071
Subtotals				247,220	316,909	371,653	935,783
Taxes - Materials Costs	@	8.75%		21,632			21,632
Subtotals				268,852	316,909	371,653	957,415
Taxes - Labor Costs	@	5.00%			15,845		15,845
Subtotals				268,852	332,755	371,653	973,260
Contractor Markup for Sub	@	12%				44,598	44,598
Subtotals				268,852	332,755	416,252	1,017,858
Contractor OH&P	@	15%		40,328	49,913		90,241
Subtotals				309,180	382,668	416,252	1,108,099
Estimate Contingency	@	30%					332,430
Subtotals							1,440,529
Escalate to Midpoint of Construct	@	3%					-
Estimated Bid Cost							1,440,529
Total Estimate							1,441,000

Estimate Accuracy			
+50%	-30%		

Estimated Range of Probable Cost					
+50%	Total Est.	-30%			
\$2,161,500	\$1,441,000	\$1,008,700			

KENNEDY/JENKS CONSULTANTS

Project:	Rancho I	Murrieta	Prepared By:		
				Date Prepared:	JLH
Building, Ar	ea:	Lookout Hill Booster Pumping Station		K/J Proj. No.	1670011*00
				Current at ENR	
Estimate Ty	pe:	Conceptual	Construction	Escalated to ENR	
		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct	
		Decian Development @	0/ Complete		

		Design Development @		_ % Comp	lete						
Spec.	Item				Materials		Instal	lation	Sub-c	ontractor	
No.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Misc Sitework	1	LS			60,000.00	60,000			60,000
							,	,			ŕ
		Pump Station Foundation(Cans)	12	CY	400.00	4,741	400.00	4,741			9,481
		Pump Station SOG	11	CY	250.00	2,778	250.00	2,778			5,556
		Generator Slab	6	CY	250.00	1,481	250.00	1,481			2,963
		Vertical Turbine Pumps	2	EA	33,002.00	66,004	8,400.00	16,800			82,804
		* Pumps outdoor, no enclosure or building included.									
		10" Butterfly Valve w/ Ext Op	2	EA	1,300.00	2,600	300.00	600			3,200
		10" FCA	2	EA	800.00	1,600	250.00	500			2,100
		Pump Discharge Piping:									
		10" Fittings/ Spools	12	EA	500.00	6,000	200.00	2,400			8,400
		10" Flex Connector	2	EA	800.00	1,600	250.00	500			2,100
		10" Check Valve	2	EA	3,700.00	7,400	250.00	500			7,900
		10" Butterfly Valve	2	EA	1,200.00	2,400	200.00	400			2,800
		10" FCA	2	EA	800.00	1,600	250.00	500			2,100
		Pipe Supports	4	EA	150.00	600	100.00	400			1,000
		CARV	2	EA	400.00	800	200.00	400			1,200
		Tee	2	EA	800.00	1,600	350.00	700			2,300
		12" Discharge Header	20	LF	60.00	1,200	25.00	500			1,700
		Pressure Gage	2	EA	250.00	500	150.00	300			800
				1.0					05.000	25.000	05.000
		Power Feed to Pump Station	1	LS					25,000	25,000	25,000
	-	Electrical / I&C	1	LS	40.000.00	00.000	0.000.00	0.000	80,000	80,000	80,000
		VFD's 50HP	2	EA	10,000.00	20,000	3,000.00	6,000			26,000
		Emergency Generator 50kW w/ ATS & Fuel Tank	1	EA	22,000.00	22,000	6,900.00	6,900			28,900
						144 004		106 400		105.000	256 204
			<u> </u>	10%		144,904		106,400		105,000	356,304
		Division 1 Costs	@	10%		14,490 159,394		10,640 117,040		10,500	35,630 391,934
		Subtotals Taxes - Materials Costs	@	8.75%		13,947		117,040		115,500	13,947
		Subtotals	w	0.75%		173,341		117,040		115,500	405,881
		Taxes - Labor Costs	@	5.00%		173,341		5,852		110,000	5,852
		Subtotals	<u>w</u>	J.UU /0		173,341		122,892		115,500	411,733
		Contractor Markup for Sub	@	12%		173,341		122,092		13,860	13,860
		Subtotals	<u> </u>	14/0		173,341		122,892		129,360	425,593
		Oubiolais				110,041	<u> </u>	122,032		123,300	+ ∠∪,∪⊎∂

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Contractor OH&P	@	15%	26,001	18,434		44,435
Subtotals			199,343	141,326	129,360	470,028
Estimate Contingency	@	30%				141,009
Subtotals						611,037
Escalate to Midpoint of Construct	@	3%				-
Estimated Bid Cost						611,037
Total Estimate						612,000

Estimate	Accuracy
+50%	-30%

Estimated Range of Probable Cost					
+50%	Total Est.	-30%			
\$918,000	\$612,000	\$428,400			

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Project:	Rancho	o Murrieta	Prepared By:		
-				Date Prepared:	JLH
Building, Area: Escuela Park Conversion - Recycled W			cled Water Irrigation Connection	K/J Proj. No.	1670011*00
				Current at ENR	
Estimate Ty	ype:	Conceptual	Construction	Escalated to ENR	
		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct	
		Design Development @	% Complete		

Spec.	Item					erials	Instal	llation	Sub-contractor		
Ño.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
	1			1.0	500.00	4 000	500.00	4.000			
		Connection Piping appurtenances	2	LS	500.00	1,000	500.00	1,000			2,000
		4" PVC Pipeline	200	LF	8.00	1,600	17.00	3,400			5,000
		Lanscaping Restoration	133	SY					10	1,333	1,333
		Paving Restoration									
		Cross Connection Testing	1	LS					1,000	1,000	1,000
						-				-	
	1	Subtotals		1		2,600		4,400		2,333	9,333
		Division 1 Costs	@	10%		260		440		233	933
		Subtotals				2,860		4,840		2,567	10,267
		Taxes - Materials Costs	@	8.75%		250					250
		Subtotals				3,110		4,840		2,567	10,517
		Taxes - Labor Costs	@	5.00%				242			242
		Subtotals				3,110		5,082		2,567	10,759
		Contractor Markup for Sub	@	12%						308	308
		Subtotals				3,110		5,082		2,875	11,067
		Contractor OH&P	@	15%		467		762			1,229
		Subtotals				3,577		5,844		2,875	12,296
		Estimate Contingency	@	30%							3,689
		Subtotals									15,984
		Escalate to Midpoint of Construct	@	3%							-
		Estimated Bid Cost									15,984
		Total Estimate									16,000

ſ	Estimate	Accuracy
Ī	+50%	-30%

Estimated	Range of Pro	bable Cost
+50%	Total Est.	-30%
\$24,000	\$16,000	\$11,200

Project:	Rand	cho Murrieta		Prepared By:	
				Date Prepared:	JLH
Building, Area: Stonehouse Park Conve			Recycled Water Irrigation Connection	K/J Proj. No.	1670011*00
				Current at ENR	
Estimate Ty	/pe:	Conceptual	Construction	Escalated to ENR	
	· .	Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct	
	i	Design Development @	% Complete		

Spec.	ltem				Mate	erials	Installation		Sub-contractor		
Ñо.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Connection Piping appurtenances	1	LS	500.00	500	500.00	500			1,000
		4" PVC Pipeline	475	LF	8.00	3,800	17.00	8,075			11,875
		Paving Removal	43	SY					10	433	433
		Paving Restoration	43	SY					75	3,250	3,250
		Lanscaping Restoration	345	SY					10	3,450	3,450
		Cross Connection Testing	1	LS					1,000	1,000	1,000
		Subtotals				4,300		8,575		8,133	21,008
		Division 1 Costs	@	10%		430		858		813	2,101
		Subtotals				4,730		9,433		8,947	23,109
		Taxes - Materials Costs	@	8.75%		414					414
		Subtotals				5,144		9,433		8,947	23,523
		Taxes - Labor Costs	@	5.00%				472			472
		Subtotals				5,144		9,904		8,947	23,995
		Contractor Markup for Sub	@	12%						1,074	1,074
		Subtotals				5,144		9,904		10,020	25,068
		Contractor OH&P	@	15%		772		1,486			2,257
		Subtotals				5,915		11,390		10,020	27,325
		Estimate Contingency	@	30%							8,198
		Subtotals									35,523
		Escalate to Midpoint of Construct	@	3%							-
		Estimated Bid Cost									35,523
		Total Estimate		[36,000

Ī	Estimate	Accuracy
	+50%	-30%

Estimated	Estimated Range of Probable Cost									
+50%	Total Est.	-30%								
\$54,000	\$36,000	\$25,200								

KENNEDY/JENKS CONSULTANTS

Project:	Rancho I	Murrieta			Prepared B						
						_	Date Prepared:	JLH			
Building, Ar	ea:	Lookout Hill Water StorageTank		_	1670011*00						
							Current at ENR				
Estimate Ty	pe:	Conceptual	Constru	ction			Escalated to ENR				
		Preliminary (w/o plans)	Change	Order		Mont	hs to Midpoint of Construct				
		Design Development @	 % Comp	olete							
Snec	ltem			Materials	Inst	allation	Sub-contractor				

Spec.	ltem				Mate	rials	Installation		Sub-c	ontractor	
Ño.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Demo Existing SteelTank	1	EA			40,000.00	40,000			40,000
		Demo Existing Tank foundation	84	CY	75.00	6,332	50.00	4,222			10,554
		New Storage Tank 200,000 gal	1	EA					135,000	135,000	135,000
		Tank Foundation	84	CY	250.00	21,108	250.00	21,108	•		42,216
		Excavation	84	CY		,	15.00	1,266			1,266
		Misc Sitework	1	LS			75,000.00	75,000			75,000
		Connection Piping Tank to Booste	1	LS	5,000.00	5,000	5,000.00	5,000			10,000
		Overflow Piping	1	LS	5,000.00	5,000	5,000.00	5,000			10,000
		Subtotals				37,440		151,596		135,000	324,036
		Division 1 Costs	@	10%		3,744		15,160		13,500	32,404
		Subtotals				41,184		166,755		148,500	356,440
		Taxes - Materials Costs	@	8.75%		3,604					3,604
		Subtotals				44,788		166,755		148,500	360,043
		Taxes - Labor Costs	@	5.00%				8,338			8,338
		Subtotals				44,788		175,093		148,500	368,381
		Contractor Markup for Sub	@	12%						17,820	17,820
		Subtotals				44,788		175,093		166,320	386,201
		Contractor OH&P	@	15%		6,718		26,264			32,982
		Subtotals				51,506		201,357		166,320	419,183
		Estimate Contingency	@	30%							125,755
		Subtotals									544,938
		Escalate to Midpoint of Construct	@	3%							
		Estimated Bid Cost									544,938
		Total Estimate									545,000

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Estimate Accuracy						
+50%	-30%					

Estimated	Estimated Range of Probable Cost									
+50%	Total Est.	-30%								
\$817,500	\$545,000	\$381,500								

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KENNEDY/JENKS CONSULTANTS

Project:	Ran	cho N	<i>N</i> urrieta		Prepared By:		
_					Date Prepared:	JLH	
Building, Area:			North Main Gate Conversion - R	ecycled Water Irrigation Connection	K/J Proj. No.	1670011*00	
					Current at ENR		
Estimate Ty	ype:		Conceptual	Construction	Escalated to ENR		
_	•		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct		
			Design Development @	% Complete			

Spec.	Item				Mate	erials	Installation		Sub-contractor		
Ño.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Connection Dining annuates and		1.0	F00.00	500	500.00	500			4 000
		Connection Piping appurtenances	200	LS	500.00	500	500.00	500			1,000
		4" PVC Pipeline	200	LF OY	8.00	1,600	17.00	3,400			5,000
		Paving Removal	33	SY			10.00	333		0.500	333
		Paving Restoration	33	SY					75	2,500	2,500
		Landscaping Restoration	33	LS					20	667	667
		Cross Connection Testing	11	LS					1,000	1,000	1,000
		Subtotals				2100.00		4233.33		4166.67	10500.00
		Division 1 Costs	@	10%		210.00		423.33		416.67	1050.00
		Subtotals				2310.00		4656.67		4583.33	11550.00
		Taxes - Materials Costs	@	8.75%		202.13					202.13
		Subtotals				2512.13		4656.67		4583.33	11752.13
		Taxes - Labor Costs	@	5.00%				232.83			232.83
		Subtotals				2512.13		4889.50		4583.33	11984.96
		Contractor Markup for Sub	@	12%						550.00	550.00
		Subtotals				2512.13		4889.50		5133.33	12534.96
		Contractor OH&P	@	15%		376.82		733.43			1110.24
		Subtotals				2888.94		5622.93		5133.33	13645.20
		Estimate Contingency	@	30%							4093.56
		Subtotals									17738.76
		Escalate to Midpoint of Construct	@	3%							
		Estimated Bid Cost									17,738.76
		Total Estimate									18,000

I	Estimate	Accuracy
	+50%	-30%

Estimated	Estimated Range of Probable Cost								
+50%	Total Est.	-30%							
\$27,000	\$18,000	\$12,600							

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OPINIO	N OF P	ROBABLE CONSTRUCTION COST	KENNEDY/JENKS CONSULTAN								
Project:	Ranch	o Murrieta						Prepared By:			
								-		Date Prepared:	JLH
Building	, Area:	Recycled Water SCADA Control System		_		K/J Proj. No.	1670011*00				
									C	Current at ENR	
Estimate	Tyne	Conceptual		Constru	ection					alated to ENR	
Lotimate		-		_			Months to Midpoint of Construct				
	L	⊋Preliminary (w/o plans)		Change	Order			Wont	ins to Milapoin	t of Construct	
		Design Development @		_ % Comp	olete						
Spec.	Item				Mate	erials	Instal	Illation Sub-contractor		contractor	
No.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
Buildout											
		Bass Lake Flow Control Valve 8" Actuated Butterfly Valve	1	EA	4,300.00	4,300	4,500.00	4,500			8,800
		Bass Lake Tank Altitude Valve 8"	1	EA	800.00	800	500.00	500			1,300
		Power Drop / Meter at Bass Lake	1	EA					25,000	25,000	25,000
		Power to Above Items	2	EA					5,000	10,000	10,000
	1	0.11.0	1	100	1			i	5.000		

Juliaout.										
	Bass Lake Flow Control Valve 8" Actuated Butterfly Valve	1	EA	4,300.00	4,300	4,500.00	4,500			8,800
	Bass Lake Tank Altitude Valve 8"	1	EA	800.00	800	500.00	500			1,300
	Power Drop / Meter at Bass Lake	1	EA					25,000	25,000	25,000
	Power to Above Items	2	EA					5,000	10,000	10,000
	Cell Communication	1	LOC					5,000	5,000	5,000
	Subtotals				5,100		5,000		40,000	50,100
	Division 1 Costs	@	10%		510		500		4,000	5,010
	Subtotals				5,610		5,500		44,000	55,110
	Taxes - Materials Costs	@	8.75%		491		-,		,	491
	Subtotals				6,101		5,500		44,000	55,601
	Taxes - Labor Costs	@	5.00%		,		275			275
	Subtotals				6,101		5,775		44,000	55,876
	Contractor Markup for Sub	@	12%						5,280	5,280
	Subtotals				6,101		5,775		49,280	61,156
	Contractor OH&P	@	15%		915		866			1,781
	Subtotals				7,016		6,641		49,280	62,937
	Estimate Contingency	@	30%							18,881
	Subtotals									81,818
	Escalate to Midpoint of Construct	@	3%							-
	Estimated Bid Cost	·								81,818
	Total Estimate									82,000

Estimate	Accuracy
+50%	-30%

Estimated Range of Probable Cost						
+50%	Total Est.	-30%				
\$123,000	\$82,000	\$57,400				

Project:	Rancho	Murrieta		Prepared By:	
				Date Prepared:	JLH
Building, Ar	ea:	Disinfection Facilties Upgrade		K/J Proj. No.	1670011*00
				Current at ENR	
Estimate Ty	pe:	Conceptual	Construction	Escalated to ENR	
		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct	
		Design Development @	% Complete		

Spec.	Item				Materials		Installation		Sub-contractor		
No.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Dana Friation 0011 00D	0.000	 			0.00	50,000			50,000
		Demo Existing 20" CCP	6,600	LF			8.00	52,800			52,800
		Demo Concrete Anchors for CCP	207	CY			150.00	30,979			30,979
	1	New Chlorine Contact Tank :		0)(40.00	4.4.400			44400
		Excavation	1,441	CY		0.1.100	10.00	14,406			14,406
		Shoring	2,440	VSF	10.00	24,400	12.00	29,280			53,680
		Base Slab	92	CY	250.00	23,111	200.00	18,489			41,600
		Tank Exterior Walls	136	CY	300.00	40,667	400.00	54,222			94,889
		Tank Center Walls	71	CY	300.00	21,333	400.00	28,444			49,778
		Backfill	516	CY			5.00	2,581			2,581
		Chlorine Injection Systems									
		Misc Sitework	1				40,000.00	40,000			40,000
	•	Subtotals				109,511	,	271,201		-	380,713
		Division 1 Costs	@	10%		10,951		27,120		-	38,071
		Subtotals				120,462		298,322		-	418,784
		Taxes - Materials Costs	@	8.75%		10,540		,			10,540
		Subtotals				131,003		298,322		-	429,324
		Taxes - Labor Costs	@	5.00%				14,916			14,916
		Subtotals		i		131,003		313,238		-	444,240
		Contractor Markup for Sub	@	12%				•		-	-
		Subtotals				131,003		313,238		-	444,240
		Contractor OH&P	@	15%		19,650		46,986			66,636
		Subtotals				150,653		360,223		-	510,876
		Estimate Contingency	@	30%				,			153,263
		Subtotals		l							664,139
		Escalate to Midpoint of Construct	@	3%							
		Estimated Bid Cost		- / -							664,139
		Total Estimate									665,000

Estimate Accuracy				
+50%	-30%			

Estimated Range of Probable Cost						
+50%	Total Est.	-30%				
\$997,500	\$665,000	\$465,500				

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KENNEDY/JENKS CONSULTANTS

Project:	Rancho	Murrieta		Prepared By:	
-				Date Prepared:	JLH
Building, A	rea:	North Golf Course Conveyance	System Rehabilitation	K/J Proj. No.	1670011*00
				Current at ENR	
Estimate Ty	ype:	Conceptual	Construction	Escalated to ENR	
_		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct	
		Design Development @	% Complete		

		Design Development @		_ % Comp	lete						
Spec.	Item		Materials			Insta	llation	Sub-	contractor		
No.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
Mostowator Boo	plamamtian	Plant to Bass Lake - 11,200 ft, of which 9,00	Oft will be imp	around \M/\M/E	D to Vollow Pride	70 (12 in 4 200 ft)	to be replaced F	Compining pine is a	pagumad ta ba 0	inch: 1/2 of which is	to be replaced th
Wasiewaler Rec		Flant to bass cake - 11,200 ft, of which 9,00	o it will be lift	Tovea. wwr	TE TO TELIOW DITUS	je (12-111, 4,300 11)	to be replaced. R		assumed to be o	-inch, 1/3 of which is	to be replaced, the
		Condition Assessment 12" AC Pip	1	LS			5,000.00	5,000	25,000	25,000	30,000
		12" PVC Pipe (100% Replaced)	4,300	LF	24.00	102 200	26.00	111,800	25,000	25,000	215,000
			4,300 36	EA	635.00	103,200 22,754	125.00	4,479			27,233
		12" Fittings	2								
		Connection to Existing Pipes		EA LF	500.00	1,000	500.00	1,000	50		2,000
		12" PVC Pipe (CIPP lined)	4.000				0.00	04.400	59		0.4.400
		Remove Existing Pipe	4,300	LF			8.00	34,400			34,400
		Paving Removal	1,911	SY			10.00	19,111			19,111
		Paving Replacement over trench	1,911	SY					75	143,333	143,333
		Traffic Controls	43	DY	200.00	8,600	1,040.00	44,720			53,320
	1										
		Condition Assessment 8" AC Pipe	1	LS			10,000.00	10,000	45,000	45,000	55,000
		O" DVC Ding Danlaged	1.000	LF	44.00	20,000	22.00	44.000		<u> </u>	60.400
		8" PVC Pipe Replaced	1,900	LF LF	14.00	26,600	22.00	41,800			68,400
		Remove Existing Pipe	1,900	SY			8.00	15,200			15,200
		Paving Removal	844				10.00	8,444	75	00.000	8,444
		Paving Replacement over trench	844	SY	222.22	0.000	4 0 40 00	40.700	75	63,333	63,333
		Traffic Controls	19	DY	200.00	3,800	1,040.00	19,760			23,560
		8" PVC Pipe (CIPP Repair)	3,800	LF					55	209,000	209,000
		Subtotals				165954.17		315,715		485,667	967,336
		Division 1 Costs	@	10%		16595.42		31,571		48,567	96,734
		Subtotals	<u> </u>	1070		182549.58		347,286		534,233	1,064,069
		Taxes - Materials Costs	@	8.75%		15973.09		347,200		334,233	15,973
		Subtotals	<u> </u>	0.7370		198522.67		347,286		534,233	1,080,042
		Taxes - Labor Costs	@	5.00%		190522.01		17,364		334,233	17,364
		Subtotals		0.0070		198522.67		364,651		534,233	1,097,407
		Contractor Markup for Sub	@	12%				,		64,108	64,108
		Subtotals				198522.67		364,651		598,341	1,161,515
		Contractor OH&P	@	15%		29778.40		54,698		,	84,476
		Subtotals				228301.07		419,348		598,341	1,245,990
		Estimate Contingency	@	30%				-,		1	373,797

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Subtotals		1,619,788
Escalate to Midpoint of Construct @ 3%		-
Estimated Bid Cost		1,619,788
Total Estimate		1,620,000

Estimate Accuracy				
+50%	-30%			

Estimated Range of Probable Cost						
+50%	Total Est.	-30%				
\$2,430,000	\$1,620,000	\$1,134,000				

Project:	Rancho	Murrieta		Prepared By:	
				Date Prepared:	JLH
Building, Ar	ea:	Bass Lake Recyled Water Stora	age Tank	K/J Proj. No.	1670011*00
				Current at ENR	
Estimate Ty	pe:	Conceptual	Construction	Escalated to ENR	
-		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct	
		Design Development @	% Complete		

Spec.	Item				Mate	erials	Instal	lation	Sub-c	contractor	
Ño.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Site Prep	1	LS			10,000.00	10,000			10,000
		One i rep		+ -5 +			10,000.00	10,000			10,000
		New Storage Tank 500,000 gal	1	EA					450,000	450,000	450,000
		Foundation	141	CY	250.00	35,180	250.00	35,180			70,359
		Overflow Piping	1	LS			10,000.00	10,000			10,000
		Misc Sitework:	1	ALL			195,000.00	195,000			195,000
											· · · · · · · · · · · · · · · · · · ·
		Subtotals				35,180		250,180		450,000	735,359
		Division 1 Costs	@	10%		3,518		25,018		45,000	73,536
		Subtotals				38,698		275,198		495,000	808,895
		Taxes - Materials Costs	@	8.75%		3,386					3,386
		Subtotals				42,084		275,198		495,000	812,281
		Taxes - Labor Costs	@	5.00%				13,760			13,760
		Subtotals				42,084		288,957		495,000	826,041
		Contractor Markup for Sub	@	12%						59,400	59,400
		Subtotals				42,084		288,957		554,400	885,441
		Contractor OH&P	@	15%		6,313		43,344			49,656
		Subtotals				48,396		332,301		554,400	935,097
		Estimate Contingency	@	30%							280,529
		Subtotals									1,215,626
		Escalate to Midpoint of Construct	@	3%							-
		Estimated Bid Cost									1,215,626
		Total Estimate									1,216,000

Estimate	Accuracy
+50%	-30%

Estimated Range of Probable Cost										
+50%	Total Est.	-30%								
\$1,824,000	\$1,216,000	\$851,200								

Project:	Rancho	o Murrieta	Prepared By:		
				Date Prepared:	JLH
Building, Are	ea:	Bass Lake Booster Pump Station		K/J Proj. No. 16700	11*00
				Current at ENR	
Estimate Typ	pe:	Conceptual	Construction	Escalated to ENR	
		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct	
		Design Development @	% Complete		

Spec.	Item				Mate			lation		contractor	
No.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Misc Sitework	1	LS			60,000.00	60,000			60,00
		Pump Station Foundation(Cans)	12	CY	400.00	4,741	400.00	4,741			9,48
		Pump Station SOG	11	CY	250.00	2,778	250.00	2,778			5,55
		Generator Slab	9	CY	250.00	2,222	250.00	2,222			4,44
		Vertical Turbine Pumps	2	EA	34,371.00	68,742	10,000.00	20,000			88,74
		* Pumps outdoor, no enclosure or building									
		10" Butterfly Valve w/ Ext Op	2	EA	1,300.00	2,600	300.00	600			3,20
		10" FCA	2	EA	800.00	1,600	250.00	500			2,10
		D D: 1 D: :									
		Pump Discharge Piping:	40	<u> </u>	500.00	0.000	000.00	0.400			0.40
		10" Fittings/ Spools	12	EA	500.00	6,000	200.00	2,400			8,40
		10" Flex Connector	2	EA	800.00	1,600	250.00	500			2,10
		10" Check Valve	2	EA	3,700.00	7,400	250.00	500			7,90
		10" Butterfly Valve	2	EA	1,200.00	2,400	200.00	400			2,80
		10" FCA	2	EA	800.00	1,600	250.00	500			2,10
		Pipe Supports	4	EA	150.00	600	100.00	400			1,00
		CARV	2	EA	400.00	800	200.00	400			1,20
		Tee	2	EA	800.00	1,600	350.00	700			2,30
		12" Discharge Header	20	LF	60.00	1,200	25.00	500			1,70
		Pressure Gage	2	EA	250.00	500	150.00	300			80
		Power Feed from Street up to Lookout Hill	1	LS					25,000	25,000	25,00
		Electrical / I&C	1	LS					80,000	80,000	80,00
		VFD's 50HP	2	EA	10,000.00	20,000	3,000.00	6,000			26,00
		Emergency Generator 50KW w/ATS and f	11	EA	22,000.00	22,000	6,900.00	6,900			28,90
		Subtotals				148,383		110,341		105,000	363,72
		Division 1 Costs	@	10%		14,838		11,034		10,500	36,37
		Subtotals				163,221		121,375		115,500	400,09
		Taxes - Materials Costs	@	8.75%		14,282					14,28
		Subtotals				177,503		121,375		115,500	414,37
		Taxes - Labor Costs	@	5.00%				6,069			6,06

Subtotals			177,503	127,444	115,500	420,446
Contractor Markup for Sub	@	12%			13,860	13,860
Subtotals			177,503	127,444	129,360	434,306
Contractor OH&P	@	15%	26,625	19,117		45,742
Subtotals			204,128	146,560	129,360	480,048 144,015
Estimate Contingency	@	30%				144,015
Subtotals						624,063
Escalate to Midpoint of Construct	@	3%				•
Estimated Bid Cost						624,063
Total Estimate						625,000

Estimate	Accuracy
+50%	-30%

Estimated	Estimated Range of Probable Cost										
+50%	Total Est.	-30%									
\$937,500	\$625,000	\$437,500									

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KENNEDY/JENKS CONSULTANTS

Project:	Rancho I	Murrieta	Prepared By:		
-				Date Prepared:	JLH
Building, Are	ea:	Seasonal Storage Reservior		K/J Proj. No.	1670011*00
				Current at ENR	
Estimate Type	oe:	Conceptual	Construction	Escalated to ENR	
		Preliminary (w/o plans)	Change Order	Months to Midpoint of Construct	
		Design Development @	% Complete		

Spec.	ltem				Mate	erials	Insta	llation	Sub-c	ontractor	
No.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Site Prep	1	LS			30,000.00	30,000			30,000
		Cut		CY			5.00				
		Import Fill & Backfill with Compact	8,750	CY	20.00	175,000	3.00	26,250			201,250
		Grading	243,442	SY			1.00	243,442			243,442
		Stormdrainage									
		Paving									
		Site Lighting									
		Fencing									
		Connection Piping									
		Overflow Piping									
		Electrical Service									
		Subtotals				175,000		299,692		-	474,692
		Division 1 Costs	@	10%		17,500		29,969		-	47,469
		Subtotals				192,500		329,661		-	522,161
		Taxes - Materials Costs	@	8.75%		16,844					16,844
		Subtotals				209,344		329,661		-	539,005
		Taxes - Labor Costs	@	5.00%				16,483			16,483
		Subtotals				209,344		346,145		-	555,488
		Contractor Markup for Sub	@	12%						-	-
		Subtotals				209,344		346,145		-	555,488
		Contractor OH&P	@	15%		31,402		51,922			83,323
		Subtotals				240,745		398,066		-	638,812
		Estimate Contingency	@	30%							191,643
		Subtotals									830,455
		Escalate to Midpoint of Construct	@	3%							-
		Estimated Bid Cost									830,455
		Total Estimate									831,000

Page 27 of 35 Date Printed 1/25/2017

Estimate	Accuracy
+50%	-30%

Estimated Range of Probable Cost							
+50%	Total Est.	-30%					
\$1,246,500	\$831,000	\$581,700					

Page 28 of 35 Date Printed 1/25/2017

KENNEDY/JENKS CONSULTANTS

Project:	Ra	ancho N	Murrieta		Prepared By:			
-							Date Prepared:	JLH
Building, Are	ea:		Van Vleck Sprayfield				K/J Proj. No.	1670011*00
							Current at ENR	
Estimate Typ	pe:		Conceptual	Constru	ction		Escalated to ENR	
			Preliminary (w/o plans)	Change	Order	Me	onths to Midpoint of Construct	
			Design Development @	 % Comp	olete			
Spec.		ltem			Materials	Installation	Sub-contractor	

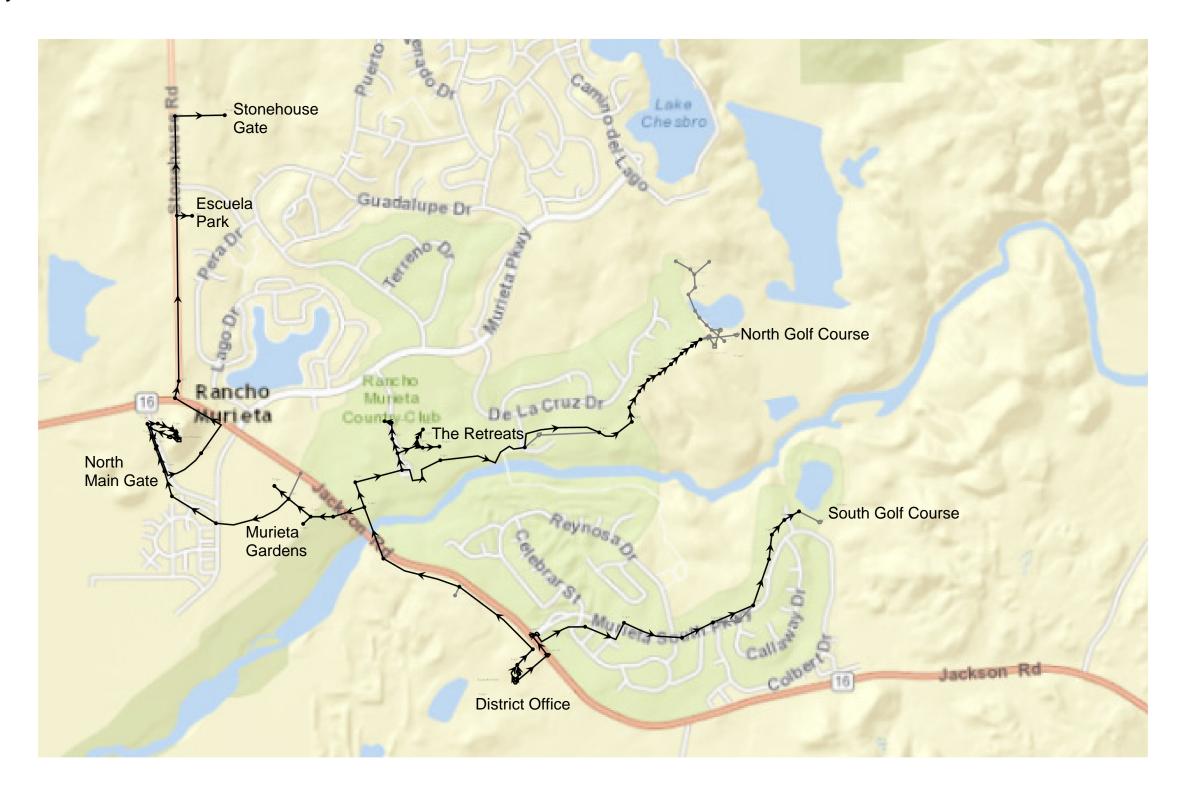
Spec.	Item				Materials		Installation		Sub-contractor		
Ñо.	No.	Description	Qty	Units	\$/Unit	Total	\$/Unit	Total	\$/Unit	Total	Total
		Above ground 12" Irrigation pipe	1,000	LF	20.08	20,075	8.91	8,910			28,985
		Above ground 8" Irrigation pipe	5,000	LF	9.90	49,500	6.27	31,350			80,850
		Above ground 6" Irrigation pipe	5,000	LF	6.44	32,175	5.21	26,070			58,245
		Above ground 4" Irrigation pipe	5,000	LF	3.34	16,720	4.33	21,670			38,390
		Above ground 4" Irrigation pipe	11,250	LF	3.34	37,620	4.33	48,758			86,378
		K Line Irrigation Systems	55	EA	2,600.00	143,000	320.00	17,600			160,600
		Valves	30	EA	1,500.00	45,000	150.00	4,500			49,500
		Outratala				0.4.4.000		450.050			500.040
		Subtotals		400/		344,090		158,858		-	502,948
		Division 1 Costs	@	10%		34,409		15,886		-	50,295
		Subtotals		0.750/		378,499		174,743		-	553,242
		Taxes - Materials Costs	@	8.75%		33,119		474740			33,119
		Subtotals		5.000/		411,618		174,743		-	586,361
		Taxes - Labor Costs	@	5.00%				8,737			8,737
		Subtotals				411,618		183,480		-	595,098
		Contractor Markup for Sub	@	12%						-	-
		Subtotals				411,618		183,480		-	595,098
		Contractor OH&P	@	15%		61,743		27,522			89,265
		Subtotals				473,360		211,002		-	684,363
		Estimate Contingency	@	30%							205,309
		Subtotals									889,672
		Escalate to Midpoint of Construct	@	3%							-
		Estimated Bid Cost									889,672
		Total Estimate									890,000

Page 29 of 35

I	Estimate	Accuracy
	+50%	-30%

Estimated Range of Probable Cost							
+50%	Total Est.	-30%					
\$1,335,000	\$890,000	\$623,000					

Rancho Murieta -Phase 1 - Proposed Recycled Water Use Conveyance System



Initial PS Capacity Estimate from Demands

- Based on meeting the demands (not including the North and South GC demands) within the 8-hr irrigation window, the pump station capacity needed is 2,955 gpm (~1480 gpm per pump, assuming 2 duty pumps).
- If Bass Lake Tank is filled outside the 8-hr irrigation period (i.e., during the hours when Bass Lake is filled for the North GC demands), then the Village A, B, and C demands can be removed from this total. The minimum RWPS capacity needed would then be **1,758 gpm** (~880 gpm per pump, assuming 2 duty pumps).
- The capacity of the RWPS is expected to be between 1,760 and 2,960 gpm.

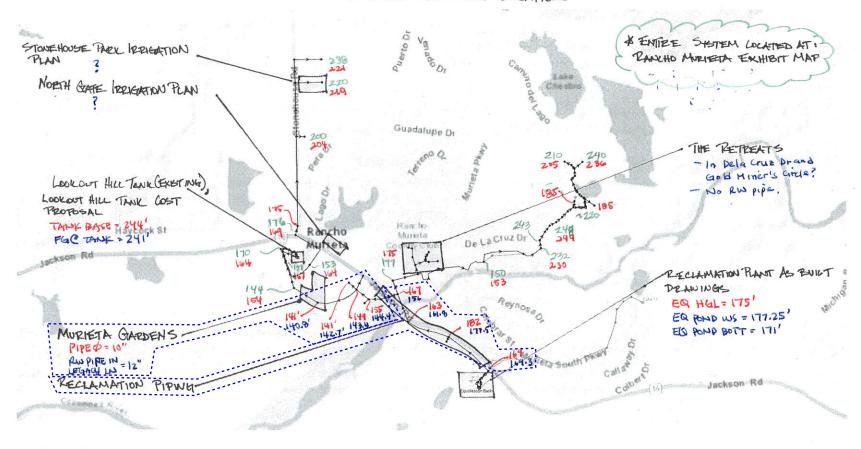
Modeling Results

- Because of pressure limitation of the pipe (criteria is to maintain pressure at Junction N_3 below 150 psi), the flow rate to Bass Lake and Bass Lake Tank is limited to ~1380 gpm. If filling Bass Lake at 1,052 gpm (North GC demand spread over 16 hrs), the maximum rate of filling Bass Lake Tank is 328 gpm (=1,380 gpm 1,052 gpm) over the 16-hr window.
- Based on the demand downstream of Bass Lake Tank, the tank would need to be filled at a rate of at least 542 gpm during the 8-hr irrigation window. Therefore the RWPS capacity needs to be at least 2,300 gpm (=1,758 gpm + 542 gpm).
- There are two design points for the RWPS, one during the 8-hr irrigation window and one during the 16-hr non-irrigation period. Here are the proposed design points:
 - o **2,600 gpm @ 195 ft** for the 8-hr period
 - o 1,400 gpm @ 345 ft for the 16-hr period

1 CHECK PHYSICAL ATTRIBUTES

Rancho Murieta Recycled Water System:

AS. BUILT DRAWING LOCATIONS



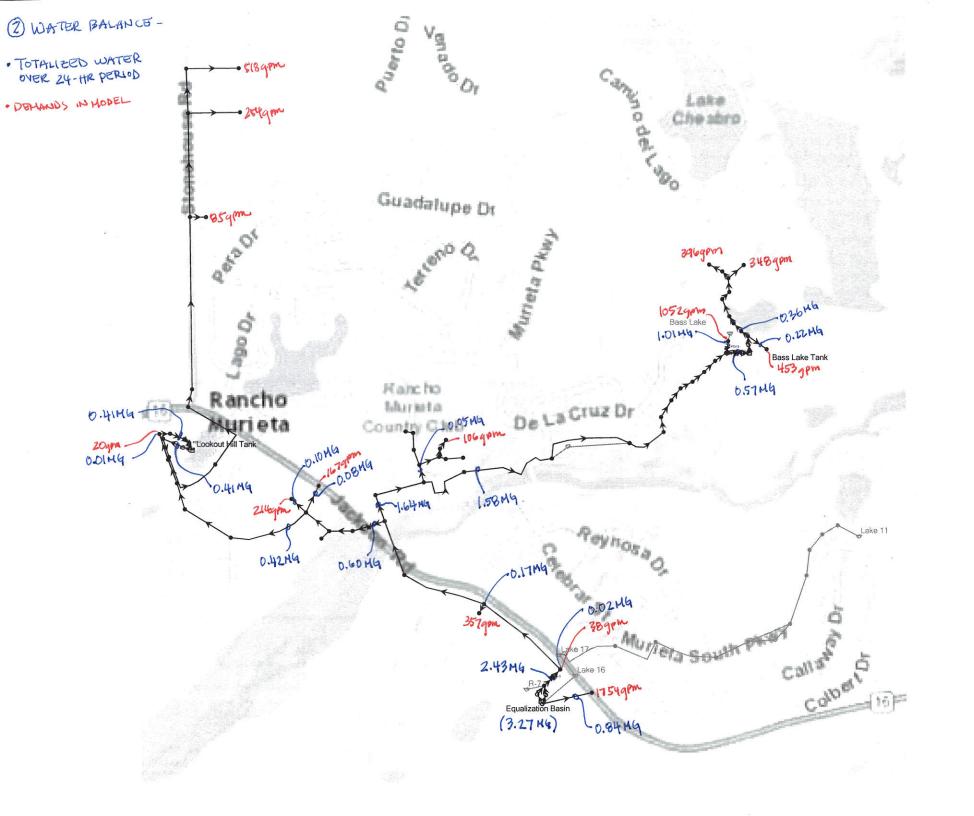
·ALL FOLDERS ARE LOCATED AT: J:/1670011*00/9.09/BACKGROUND INFORMATION

RMCSD RW Model.wtg 10/11/2016 Bentley Systems, Inc. Haestad Methods Solution Center 27 Siemon Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666 Bentley WaterCAD V8i (SELECTseries 6) [08.11.06.113]

- Model

- prawing

- Exhibit Map



G3 Engineering, Inc.



06 Dec 2016

Kennedy Jenks Quotation number: 480863

Sacramento, CA Revision:

Attn: Ryan Young

Project: Rancho Murieta

Your reference:

We thank you for your above referenced inquiry, and are pleased to submit our quotation for your consideration.

The following is a budget price summary for this quotation. Please see item specific pages for more details.

Item number	Service	Size	Unit Price	Unit Freight	Qty	Extended Price
010	RW Booster PS (1480 GPM)	14DOL - 5 stage Product lube - Sump Pump	\$ 46,167	\$ 1,000	3	\$ 141,501
011	Lookout Hill BPS (860 GPM)	11JKH - 2 stage Product lube - Barrel Pump	\$ 32,002	\$ 1,000	2	\$ 66,004
012	Bass Lake BPS (1200 GPM)	12JKH - 2 stage Product lube - Barrel Pump	\$ 33,371	\$ 1,000	2	\$ 68,742
Grand Total						

COMMENTS:

- a. Pricing is for budget purposes only.
- b. Quote does not include: Installation, Oil or Grease, Valves, Gauges, Anchor Bolts, Soleplates, Spare Parts, Sales Tax.

SHIPMENT AND FREIGHT TERMS: Shipment is quoted with freight term: Per the freight term listed in the Comments and Clarifications Section. Partial shipment allowed. Shipment & invoicing will occur upon shipment of equipment. Shipment schedules are based on factory loading at time of order. Should shipment be postponed due to project or site delays Weir Floway will invoice and hold the shipment. Shipment delays exceeding 30 days from the completed date may be subject to reasonable storage charges.

LEADTIME: Submittal will be approximately 6-8 weeks after order receipt, contingent upon order acceptance within 10 business days of receipt. Orders will be accepted subject to buyer's credit approval and subject to Weir Floway, Inc.'s Terms and Conditions of Sale.

Shipment lead time will be approximately 20-22 weeks after written release to manufacture. Shipment lead times are an estimate at time of quotation and subject to change based on quote validity.

SCOPE OF SUPPLY: Please note any requirements not outlined in the referenced specification sections as noted on the cover page of this quotation will not be the responsibility of Weir Floway. Any separate specifications made reference to within the noted specifications, whether in part or whole, will not be considered in this quotation.

Weir Floway, Inc. Terms and Conditions of Sale per attached will apply to this quotation. If this is not acceptable, mutually agreeable terms and conditions may be negotiated at time of order placement.

G3 Engineering, Inc.



SPECIFICATIONS: Written request. No detailed specifications received.

VALIDITY: This offer is valid for 30 days from date issued. Quoted prices will be held firm thru shipment if order is released for manufacture within 60 days from order entry date. Otherwise, a price adjustment may be applied.

In the event that Weir Floway, Inc. is successful in the tender based on this Scope Letter, please issue the formal Purchase Order to the following address:

Weir Floway, Inc. 2494 S. Railroad Ave. Fresno, CA 93706

PRICE: Quoted prices will be held firm through shipment if order is released for manufacture within 60 days from order entry date, and approved for shipment within the leadtime quoted. Otherwise, a price adjustment may be applied. Price quoted is for all items purchased and shipped at one time. In the event of a partial order, we will review and adjust the freight price accordingly. Freight charges will be those in effect at time of shipment. Due to volatility in the commodities markets, Weir Floway reserves the right to add a material surcharge on pipe, plate, and other materials in line with the commodity indices. Cost surcharges must be agreed to prior to order acceptance.

PAYMENT TERMS: Orders & contracts are subject to approval by Weir Floway prior to acceptance. Standard terms for orders <= \$150,000 are net thirty (30) days from date of invoice. For orders >=\$150,000, progress payments will apply. Weir Floway's standard progress payment schedule is attached for consideration. Start-up services are included and will be invoiced when services are completed or eight (8) weeks from pump shipment which ever occurs first.

PACKAGING: For domestic shipment via commercial carrier. Export boxing and documentation requirements are an option with price adder.

START-UP: Start-up/assistance by authorized Rep. included. Invoice for start-up services will be issued when services are complete or 8 weeks from pump shipment whichever occurs first.

QUALITY STANDARDS: All our manufacturing locations are ISO 9001-2008 certified.

TERMS AND CONDITIONS: This quotation is based solely upon the terms and conditions set forth herein including attachments. They supersede and reject any conflicting terms and conditions of Purchaser. Any other terms and conditions that Purchaser may propose are subject to requotation.

We hope you find our quotation in line with your requirements. However, if you have any questions, please do not hesitate to contact us.

Sincerely,

Mike Burns G3 Engineering, Inc.

CC: Jim Billings, G3 Engineering

G3 Engineering, Inc.



Richard Plitt, Floway

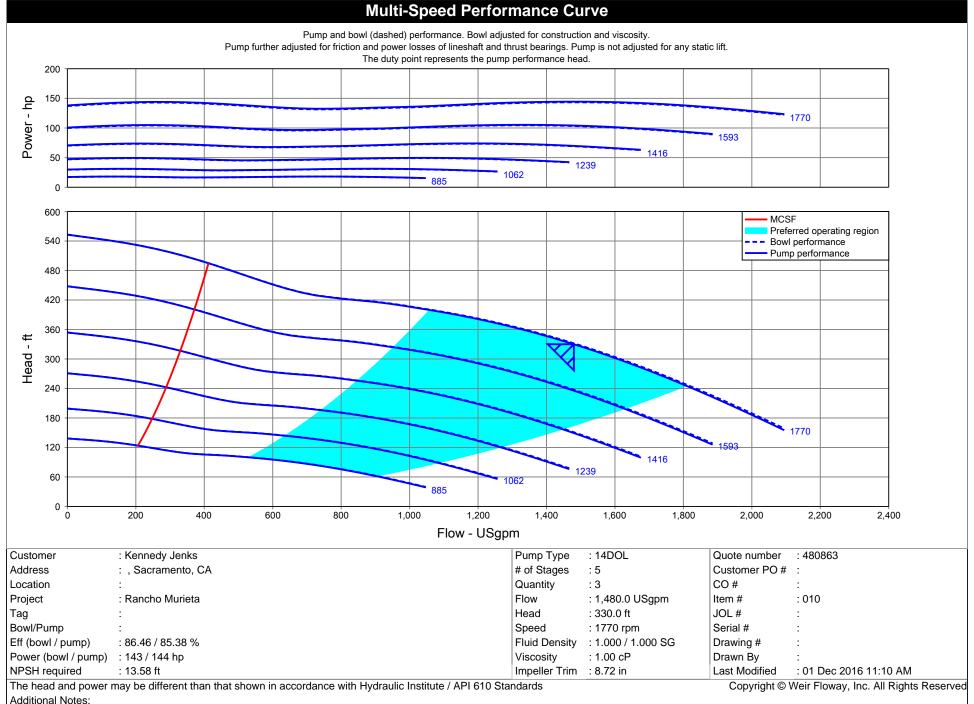
www.g3engineering.com



Pump Performance Datasheet Quote number Customer : Kennedy Jenks : 480863 Customer reference Size : 14DOL : 010 Item number Stages : 5 : RW Booster PS (1480 GPM) : 14DOL 1770 Rev. 0 Service Based on curve number Date last saved Quantity : 01 Dec 2016 11:10 AM Liquid **Operating Conditions** : 1,480.0 USgpm Liquid type : Water - Potable Flow, rated Differential head / pressure, rated (requested) : 330.0 ft Additional liquid description Differential head / pressure, rated (actual) : 331.3 ft Solids diameter, max : 0.00 in Suction pressure, rated / max : 0.00 / 0.00 psi.g Solids concentration, by volume : 0.00 % Solids concentration, by weight NPSH available, rated : 0.00 % : Ample : 60 Hz Frequency Temperature, max : 68.00 deg F : 1.000 / 1.000 SG Performance Fluid density, rated / max : 1.00 cP Speed, rated : 1770 rpm Viscosity, rated Vapor pressure, rated : 0.00 psi.a Impeller diameter, rated : 8.72 in Material Impeller diameter, maximum : 9.13 in Impeller diameter, minimum : 6.81 in Material selected : Cast Iron/Bronze : 86.46 / 85.38 % Efficiency (bowl / pump) Pressure Data NPSH required / margin required : 13.58 / 0.00 ft Maximum working pressure : See the Additional Data page : 2,908 / 9,030 US Units Ns (imp. eye flow) / Nss (imp. eye flow) Component pressure limit : See the Additional Data page MCSF : 412.4 USgpm Maximum allowable suction pressure Head, maximum, rated diameter : 553.0 ft Hydrostatic test pressure : See the Additional Data page Head rise to shutoff (bowl / pump) : 66.70 / 67.56 % **Driver & Power Data** : 1,509.2 / 1,496.6 USgpm Driver sizing specification Flow, best eff. point (bowl / pump) : Max power + 4% Flow ratio, rated / BEP (bowl / pump) : 98.07 / 98.89 % Margin over specification : 0.00 % Diameter ratio (rated / max) : 95.55 % Service factor : 1.15 Head ratio (rated dia / max dia) : 89.62 % Power, hydraulic : 124 hp Cq/Ch/Ce/Cn [ANSI/HI 9.6.7-2010] : 1.00 / 1.00 / 1.00 / 1.00 : 143 / 144 hp Power (bowl / pump) Selection status : Acceptable Power, maximum, rated diameter : 144 hp : 200 hp / 149 kW Minimum recommended motor rating Pump and bowl (dashed) performance. Bowl adjusted for construction and viscosity. Pump further adjusted for friction and power losses of lineshaft and thrust bearings. Pump is not adjusted for any static lift. The duty point represents the pump performance head. 200 Power - hp 150 Power 100 50 0 800 100 Preferred operating region 90 720 Bowl performance Pump performance 9.13 in 80 640 560 70 Efficiency 8.72 in 480 60 Head - ft Efficiency 50 400 6.81 in 320 240 30 160 20 80 10 0 0 NPSHr - ft 50 25 0 1,200 200 800 1,000 1,400 1,600 400 600 1.800 2.000 2.200 2.400 Flow - USgpm

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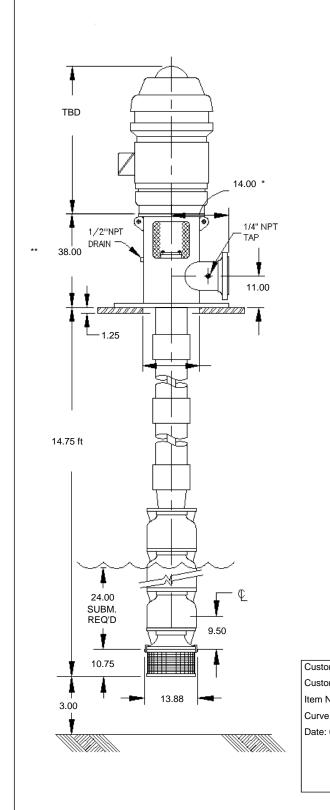


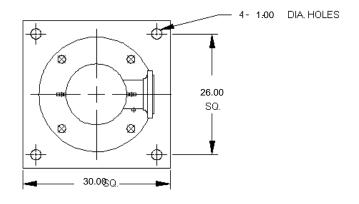


G3 Engineering, Inc. Quote No. 480863



VERTICAL TURBINE PUMP 1,480.0 USgpm 331.7 ft TDH 5 STAGE TYPE 14DOL 10x16.5F DISCHARGE HEAD





Discharge

10 in. 150#RF - ANSI Flange

16 in. Dia. Flange

12 - 1 in. Dia. holes

14.25 in. Bolt circle

- * TYPICAL LOCATION FOR DISCHARGE NOZZLE
- ** FINAL HEAD HEIGHT WILL BE DETERMINED BASED ON INTERNAL ANALYSIS AND SPECIFICATION REVIEW

NOT TO BE USED FOR CONSTRUCTION UNLESS CERTIFIED.

		DIMEN	SIONS IN NOT TO S	INCHES UNLESS OTHERWIS	SE NOTED.
	REV.	BY	DATE	DESCRIP	TION
mer: Ker	nedy	Jenks			0.17.10.5
mer Refe	rence:				OUTLINE DRAWING
lumber: 0	01				DRAWING
Number:	14DC	L 1770			
01 Dec 2	016				
					DRAWING

Customer Technical Offer

Weir Floway Inc. SCORE 16.5.1.0



Customer: Kennedy Jenks Reference:

 Item number
 010
 Size / Stages
 14DOL / 5

 Quote number
 480863
 Nominal pump speed
 1770 rpm

Totals

Grand Total \$ 141,502

Pump

Qty Description

3 Units - 14DOL - 5 stage Product lube - Sump Pump

Pump selection criteria

Speed operation: Variable speed operation

Lubrication type

Lubrication type: Product lube Bowl Assembly - 5 Stage

Bowl size: 14DOL bowl assembly - 5 stage

Bowl Materials: Cast iron (ASTM A48 cl 30-enamel lined)

Bowl connection type: Flanged

Bowl Bolting Material: 304SS (ASTM F593 Gr CW1), Floway material code - 106

Bowl bearing material: Bismuth tin bronze bowl bearings (UNS C89835)

Impeller Material: Bronze (ASTM B584 C90300) Collet Material: Steel (ASTM A108-90a Gr 1215)

Bowl Shaft Size: 1.9375" (Standard)

Bowl Shaft Material: 416SS (ASTM A582-88a Type 416)

Suction type: Suction bell

Suction type bearing: Bismuth tin bronze (UNS C89835)

Suction Strainer: Clip on basket strainer 14DO

Suction Strainer Material

Strainer material - Galvanized steel

Bowl assembly type: Fully assembled

Column assembly - 1.5 x 10 in. - Threaded

Column

Column Size: Column 10" - (0- 20' and 0- 10' and 1- 5' and 1 - 2.58' Top)

Column pipe material: ASTM A53 Gr. B rolled and welded steel Column pipe schedule: Floway standard .279" wall thickness

Column Connection Type: Threaded

Bearing Retainer material: Ductile iron (ASTM A536-84 Gr 60-40-18)

Lineshaft

Lineshaft Size: 1.5"

Lineshaft Material: 416SS (ASTM A582-88a Type 416)

Lineshaft Coupling Material: 416SS (ASTM A582-88a Type 416)

Line shaft bearing material: Styrene Butadiene Rubber(SBR) (Qty 1 per pump)

Discharge head assembly - 10x16.5 "F"

Discharge head material: Steel (A36 plt, A105 flg, A53-Gr B pipe)

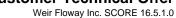
Discharge Head Size: 10x16.5 "F"

Discharge size: 10"

Discharge Connection Type/Rating: 150# flange (Stl. std.)

Shaft sealing arrangement: Mechanical seal

Mechanical seal construction: Single unbalanced mechanical seal





Customer: Kennedy Jenks **Reference**:

Pump

Qty Description

Mechanical seal type: John Crane type 5611 mechanical seal Seal flush piping plan-Primary: Plan 13 Seal flush piping

Seal flush piping material - primary seal: 316SS tubing-Primary SFP

Top Line Shaft Straightness: Floway Standard

Stuffing box / Seal housing bearing material: Bismuth tin bronze seal housing bearing (UNS C89835)

Head shaft couplings: Type CPAT flanged adjustable spacer coupling

Coupling guard material / construction: Aluminum

Protective coatings

Protective coating - Discharge head: Carboguard 891 epoxy coating - Disch. head - interior and exterior

Protective coating - Column: Carboguard 891 epoxy coating - Column - interior and exterior Protective coating - Bowl assembly: Carboguard 891 epoxy coating - Bowls, exterior only Protective coating - Soleplate: Carboguard 891 epoxy coating - Soleplate top side only

Miscellaneous coating options

NSF certified

Assembly type - Unit

Assembly type - Unit: Factory assembled (bowl, head, and column only) shipped assembled

Start-up/Overage

Start-up options

Start up by Distributor/Manufacturer's Rep.

Packaging and Shipping

Packaging options

Domestic packaging

Testing

Qty Description

3 Testing and Inspection options

Performance / NPSH testing

Factory performance test acceptance criteria for rated condition per: ANSI/HI 14.6 grade 1U (Floway standard)

Performance test options

Bowl assembly performance test - 3 units

Performance test witnessing

Non-witnessed

Hydro testing

Hydrotest - Discharge Head options: Non witnessed hydrotest - discharge head - 3 units

Inspection and Analysis

Analysis

Seismic analysis of anchorage

Structural natural frequency analysis (head/motor only), stamped by Floway P.E. - 1 units

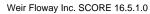
Sole Plate

Qty Description

B Discharge head assembly - 10x16.5 "F"

Soleplate type: Fabricated steel Soleplate size: 30"x30"x1.25"

Customer Technical Offer





Customer: Kennedy Jenks **Reference**:

Anchor Bolt

Qty Description

3 Discharge head assembly - 10x16.5 "F"

Soleplate anchor bolts with nuts: No soleplate anchor bolts

Driver

Qty Description

3 Driver

Electric motor driver

Motor size selection: US 200HP 460v/3ph/60hz 1800 RPM WPI

Motor efficiency type: Premium efficient

Motor shaft

Motor shaft type: Motor vertical solid shaft

Reference head shaft diameter: For reference:1.5" Top line shaft diameter

Motor thrust design High thrust

Motor bearing life options: 1 yr. min. / 5 yr. average

Motor enclosure: WPI Motor service factor: 1.15

Starting method: Across the line starting

Motor BD: Motor BD 16.5 in. Miscellaneous motor options

Thermostats
Inverter duty motor

Non-reverse device: No non-reverse device on motor

Motor testing options

Motor complete test - unwitnessed Conduit box size: Standard conduit box Elevation: Motor suitable for elevation <= 3300'

Ambient temperature: Motor suitable for ambient temperature <= 104 F (40 C)

UL labeled motor: Not UL labeled

Motor packaging options: Motor domestic packaging

Driver design: NEMA

Driver shipping options: Motor NOT to be shipped to Floway factory

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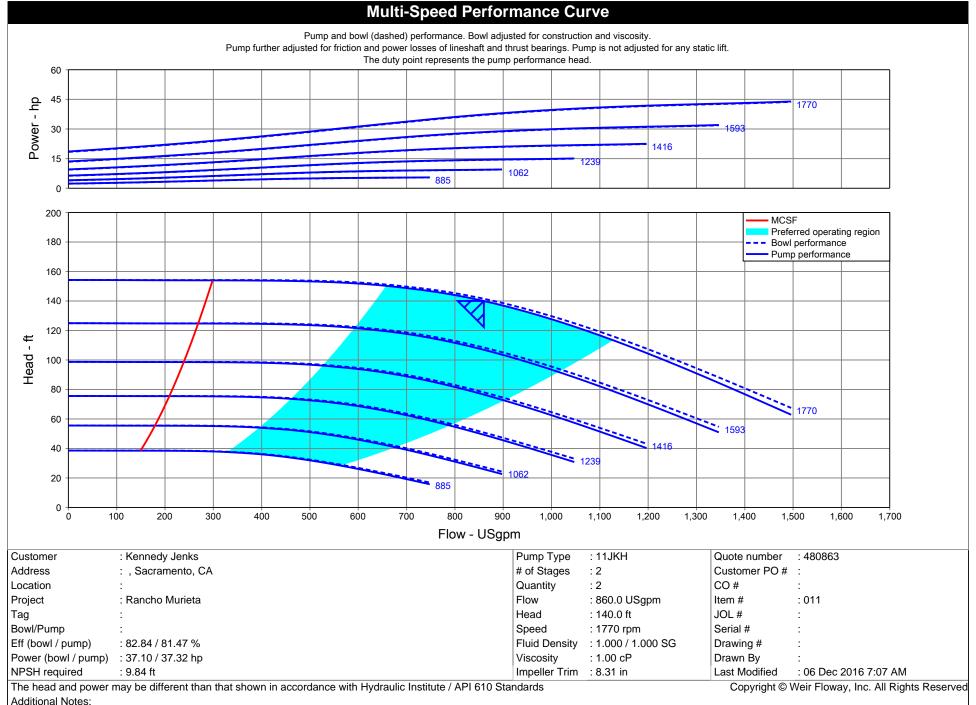


Pump Performance Datasheet Quote number Customer : Kennedy Jenks : 480863 Customer reference Size : 11JKH Item number : 011 Stages : 2 : Lookout Hill BPS (860 GPM) : 11JKH 1770 Rev. 0 Service Based on curve number Date last saved Quantity : 06 Dec 2016 7:07 AM Liquid **Operating Conditions** : 860.0 USgpm Liquid type : Water - Potable Flow, rated Differential head / pressure, rated (requested) : 140.0 ft Additional liquid description Differential head / pressure, rated (actual) : 141.1 ft Solids diameter, max : 0.00 in Suction pressure, rated / max : 0.00 / 0.00 psi.g Solids concentration, by volume : 0.00 % Solids concentration, by weight NPSH available, rated : 0.00 % : Ample : 60 Hz Frequency Temperature, max : 68.00 deg F : 1.000 / 1.000 SG Performance Fluid density, rated / max : 1.00 cP Speed, rated : 1770 rpm Viscosity, rated Vapor pressure, rated : 0.00 psi.a Impeller diameter, rated : 8.31 in Material Impeller diameter, maximum : 8.31 in Impeller diameter, minimum : 7.20 in Material selected : Cast Iron/Bronze : 82.84 / 81.47 % Efficiency (bowl / pump) Pressure Data NPSH required / margin required : 9.84 / 0.00 ft Maximum working pressure : See the Additional Data page Ns (imp. eye flow) / Nss (imp. eye flow) : 2,285 / 8,978 US Units Component pressure limit : See the Additional Data page MCSF : 298.4 USgpm Maximum allowable suction pressure Head, maximum, rated diameter : 154.3 ft Hydrostatic test pressure : See the Additional Data page Head rise to shutoff (bowl / pump) : 9.00 / 10.18 % Driver & Power Data Flow, best eff. point (bowl / pump) : 939.7 / 921.6 USgpm : Max power + 4% Driver sizing specification Flow ratio, rated / BEP (bowl / pump) : 91.52 / 93.31 % Margin over specification : 0.00 % Diameter ratio (rated / max) : 100.00 % Service factor : 1.15 Head ratio (rated dia / max dia) : 99.22 % Power, hydraulic : 30.74 hp Cq/Ch/Ce/Cn [ANSI/HI 9.6.7-2010] : 1.00 / 1.00 / 1.00 / 1.00 : 37.10 / 37.32 hp Power (bowl / pump) Selection status : Acceptable Power, maximum, rated diameter : 43.94 hp : 50.00 hp / 37.29 kW Minimum recommended motor rating Pump and bowl (dashed) performance. Bowl adjusted for construction and viscosity. Pump further adjusted for friction and power losses of lineshaft and thrust bearings. Pump is not adjusted for any static lift. The duty point represents the pump performance head. 60 Power - hp 45 Powe 30 15 0 200 100 MCSF Preferred operating region 90 180 Bowl performance Pump performance 80 160 140 8.31 ir 70 7.20 in 120 60 Head - ft Efficiency Efficiency 50 100 80 60 30 40 20 20 10 Λ NPSHr - ft 40 20 0 200 300 400 500 600 700 800 900 1,000 1,100 1,200 1,300 1.400 1.500 Flow - USgpm

G3 Engineering, Inc. Quote No. 480863

www.g3engineering.com

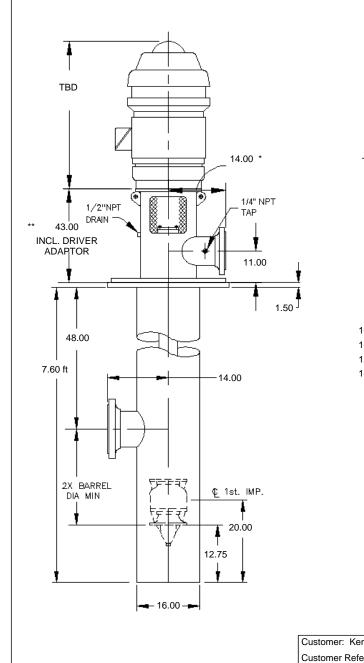


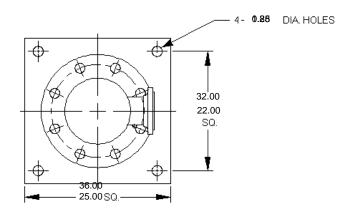


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VERTICAL TURBINE PUMP 860.0 USgpm 141.5 ft TDH 2 STAGE TYPE 11JKH 10x16.5x16VF DISCHARGE HEAD





Discharge

10 in. 150#RF - ANSI Flange 10 in. 150#RF - ANSI Flange

 16 in. Dia. Flange
 16 in. Dia. Flange

 12 - 1 in. Dia. holes
 12 - .75 in. Dia. holes

 14.25 in. Bolt circle
 14.25 in. Bolt circle

* TYPICAL LOCATION FOR DISCHARGE NOZZLE

DATE

REV. BY

** FINAL HEAD HEIGHT WILL BE DETERMINED BASED ON INTERNAL ANALYSIS AND SPECIFICATION REVIEW

Suction

NOT TO BE USED FOR CONSTRUCTION UNLESS CERTIFIED.

DESCRIPTION

NOTES: ALL DIMENSIONS IN INCHES UNLESS OTHERWISE NOTED. DRAWING NOT TO SCALE.

Customer: Ker	nedy .	Jenks		
Customer Refe	rence:			OUTLINE
Item Number: 0	11			DRAWING
Curve Number:	11JKI	H 1770		
Date: 06 Dec 2	016			
				DRAWING

Customer Technical Offer Weir Floway Inc. SCORE 16.5.1.0



Customer: Kennedy Jenks Reference:

Item number011Size / Stages11JKH / 2Quote number480863Nominal pump speed1770 rpm

Totals

Grand Total \$66,004

Pump

Qty Description

2 Units - 11JKH - 2 stage Product lube - Barrel Pump

Pump selection criteria

Speed operation: Variable speed operation

Lubrication type

Lubrication type: Product lube

Bowl Assembly - 2 Stage

Bowl size: 11JKH bowl assembly - 2 stage

Bowl Materials: Cast iron (ASTM A48 cl 30-enamel lined)

Bowl connection type: Flanged

Bowl Bolting Material: 304SS (ASTM F593 Gr CW1), Floway material code - 106

Bowl bearing material: Bismuth tin bronze bowl bearings (UNS C89835)

Impeller Material: Bronze (ASTM B584 C90300) Collet Material: Steel (ASTM A108-90a Gr 1215)

Bowl Shaft Size: 1.6875" (Standard)

Bowl Shaft Material: 416SS (ASTM A582-88a Type 416)

Suction type: Suction bell

Suction type bearing: Bismuth tin bronze (UNS C89835)

Suction Strainer: Clip on basket strainer 11JK

Suction Strainer Material

Strainer material - Galvanized steel

Bowl assembly type: Fully assembled Column assembly - 1 x 8 in. - Threaded

Column

Column Size: Column 8" - (0- 20' and 0- 10' and 0- 5' and 1 - 3.48' Top)

Column pipe material: ASTM A53 Gr. B rolled and welded steel Column pipe schedule: Schedule 30 .277" wall thickness

Column Connection Type: Threaded

Lineshaft

Lineshaft Size: 1"

Lineshaft Material: 416SS (ASTM A582-88a Type 416)

Lineshaft Coupling Material: 416SS (ASTM A582-88a Type 416)

Line shaft bearing material: Styrene Butadiene Rubber(SBR) (Qty 0 per pump)

Discharge head assembly - 10x16.5x16 "VF"

Discharge head material: Steel (A36 plt, A105 flg, A53-Gr B pipe)

Discharge Head Size: 10x16.5x16 "VF"

Discharge size: 10"

Discharge Connection Type/Rating: 150# flange (Stl. std.)

Shaft sealing arrangement: Mechanical seal

Mechanical seal construction: Single unbalanced mechanical seal Mechanical seal type: John Crane type 5611 mechanical seal



Customer: Kennedy Jenks **Reference**:

Weir Floway Inc. SCORE 16.5.1.0

Pump

Qty Description

Seal flush piping plan-Primary: Plan 13 Seal flush piping

Seal flush piping material - primary seal: 316SS tubing-Primary SFP

Top Line Shaft Straightness: Floway Standard

Stuffing box / Seal housing bearing material: Bismuth tin bronze seal housing bearing (UNS C89835)

Head shaft couplings: Type CPAT flanged adjustable spacer coupling

Coupling guard material / construction: Aluminum

Protective coatings

Protective coating - Discharge head: Carboguard 891 epoxy coating - Disch. head - interior and exterior

Protective coating - Column: Carboguard 891 epoxy coating - Column - interior and exterior Protective coating - Bowl assembly: Carboguard 891 epoxy coating - Bowls, exterior only

Protective coating - Barrel: Carboguard 891 epoxy coating - Barrel - interior only (exterior Carboline 635 primer)

Protective coating - Soleplate: Carboguard 891 epoxy coating - Soleplate top side only

Miscellaneous coating options

NSF certified

Assembly type - Unit

Assembly type - Unit: Factory assembled (bowl, head, and column only) shipped assembled

Start-up/Overage

Start-up options

Start up by Distributor/Manufacturer's Rep.

Packaging and Shipping

Packaging options

Domestic packaging

Testing

Qty Description

2 Testing and Inspection options

Performance / NPSH testing

Factory performance test acceptance criteria for rated condition per: ANSI/HI 14.6 grade 1U (Floway standard)

Performance test options

Bowl assembly performance test - 2 units

Performance test witnessing

Non-witnessed

Hydro testing

Hydrotest - Discharge Head options: Non witnessed hydrotest - discharge head - 2 units Hydrotest - Suction barrel options: Non witnessed hydrotest - suction barrel - 2 units

Inspection and Analysis

Analysis

Seismic analysis of anchorage

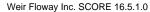
Structural natural frequency analysis (head/motor only), stamped by Floway P.E. - 1 units

Sole Plate

Qty Description

2 Discharge head assembly - 10x16.5x16 "VF"

Soleplate type: Fabricated steel Soleplate size: 36"x36"x1.25"





Customer: Kennedy Jenks **Reference**:

Anchor Bolt

Qty Description

2 Discharge head assembly - 10x16.5x16 "VF"

Soleplate anchor bolts with nuts: No soleplate anchor bolts

Barrel

Qty Description

2 Suction barrel 16 in. x 7.6 ft.

Suction barrel: Standard pressure suction barrel
Barrel diameter: 16" diameter suction barrel x 7.6 ft.
Barrel material: Steel barrel - ASTM A53 pipe A240 plate
Barrel suction nozzle: 10" suction nozzle and flange on barrel

Barrel suction flange rating: 150# suction flange

Driver

Qty Description

2 Driver

Electric motor driver

Motor size selection: US 50HP 460v/3ph/60hz 1800 RPM WPI

Motor efficiency type: Premium efficient

Motor shaft

Motor shaft type: Motor vertical solid shaft

Reference head shaft diameter: For reference:1" Top line shaft diameter

Motor thrust design High thrust

Motor bearing life options: 1 yr. min. / 5 yr. average

Motor enclosure: WPI Motor service factor: 1.15

Starting method: Across the line starting

Motor BD: Motor BD 12 in. Miscellaneous motor options

Thermostats
Inverter duty motor

Non-reverse device: No non-reverse device on motor

Motor testing options

Motor complete test - unwitnessed Conduit box size: Standard conduit box Elevation: Motor suitable for elevation <= 3300'

Ambient temperature: Motor suitable for ambient temperature <= 104 F (40 C)

UL labeled motor: Not UL labeled

Motor packaging options: Motor domestic packaging

Driver design: NEMA

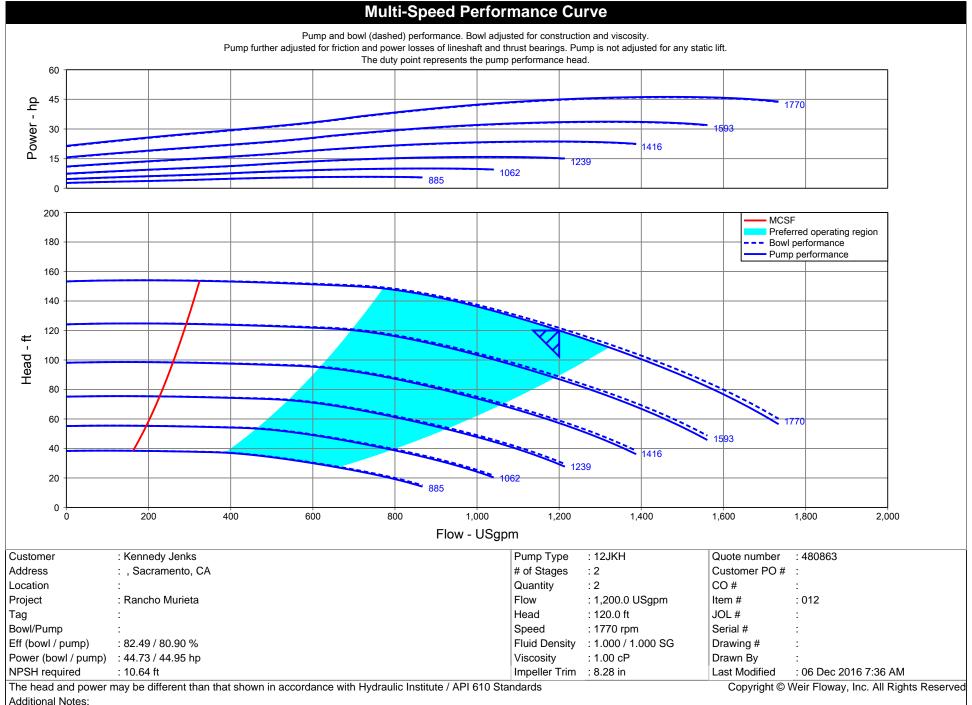
Driver shipping options: Motor NOT to be shipped to Floway factory



Pump Performance Datasheet Quote number Customer : Kennedy Jenks : 480863 Customer reference Size : 12JKH : 012 Item number Stages : 2 : Bass Lake BPS (1200 GPM) : 12JKH 1770 Rev. 0 Service Based on curve number Date last saved Quantity : 06 Dec 2016 7:36 AM Liquid **Operating Conditions** Flow, rated : 1,200.0 USgpm Liquid type : Water - Potable Differential head / pressure, rated (requested) : 120.0 ft Additional liquid description Differential head / pressure, rated (actual) : 121.2 ft Solids diameter, max : 0.00 in Suction pressure, rated / max : 0.00 / 0.00 psi.g Solids concentration, by volume : 0.00 % NPSH available, rated Solids concentration, by weight : 0.00 % : Ample : 60 Hz Frequency Temperature, max : 68.00 deg F : 1.000 / 1.000 SG Performance Fluid density, rated / max : 1.00 cP Speed, rated : 1770 rpm Viscosity, rated Impeller diameter, rated Vapor pressure, rated : 0.00 psi.a : 8.28 in Material Impeller diameter, maximum : 9.06 in Impeller diameter, minimum : 7.69 in Material selected : Cast Iron/Bronze Efficiency (bowl / pump) : 82.49 / 80.90 % Pressure Data NPSH required / margin required : 10.64 / 0.00 ft Maximum working pressure : See the Additional Data page : 2,348 / 10,219 US Units Ns (imp. eye flow) / Nss (imp. eye flow) Component pressure limit : See the Additional Data page MCSF : 324.3 USgpm Maximum allowable suction pressure Head, maximum, rated diameter : 154.1 ft Hydrostatic test pressure : See the Additional Data page Head rise to shutoff (bowl / pump) : 25.88 / 27.74 % **Driver & Power Data** : 1,101.5 / 1,075.2 USgpm Driver sizing specification Flow, best eff. point (bowl / pump) : Max power + 4% Flow ratio, rated / BEP (bowl / pump) : 108.94 / 111.61 % Margin over specification : 0.00 % Diameter ratio (rated / max) : 91.44 % Service factor : 1.15 Head ratio (rated dia / max dia) : 75.44 % Power, hydraulic : 36.90 hp Cq/Ch/Ce/Cn [ANSI/HI 9.6.7-2010] : 1.00 / 1.00 / 1.00 / 1.00 : 44.73 / 44.95 hp Power (bowl / pump) Selection status : Acceptable Power, maximum, rated diameter : 46.26 hp : 50.00 hp / 37.29 kW Minimum recommended motor rating Pump and bowl (dashed) performance. Bowl adjusted for construction and viscosity. Pump further adjusted for friction and power losses of lineshaft and thrust bearings. Pump is not adjusted for any static lift. The duty point represents the pump performance head. 60 Power - hp 45 Power 30 15 0 250 100 Preferred operating region 90 225 Bowl performance Pump performance 80 200 9.06 in 175 70 8 28 in 150 60 Head - ft 7.69 in Efficiency 50 125 100 40 75 30 50 20 25 10 Λ 0 NPSHr - ft 40 NPSHr 20 0 800 200 600 1,000 1,200 1,400 400 1,600 1.800 2.000 2.200 Flow - USgpm

www.g3engineering.com

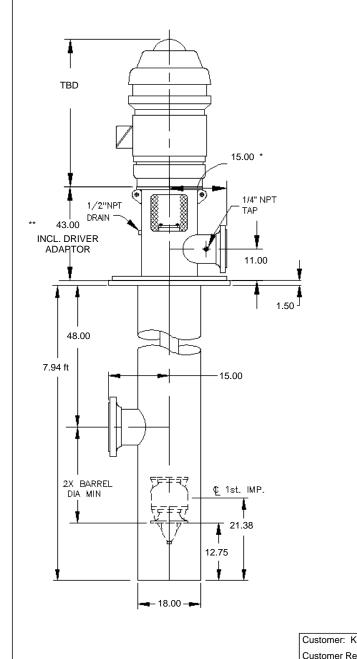


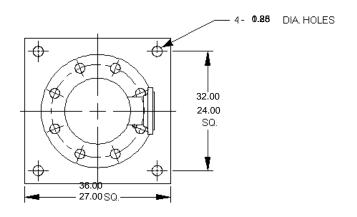


G3 Engineering, Inc. Quote No. 480863



VERTICAL TURBINE PUMP 1,200.0 USgpm 121.8 ft TDH 2 STAGE TYPE 12JKH 10x16.5x18VF DISCHARGE HEAD





Discharge

DATE

REV. BY

Suction

10 in. 150#RF - ANSI Flange

12 in. 150#RF - ANSI Flange

DESCRIPTION

16 in. Dia. Flange

19 in. Dia. Flange 12 - .75 in. Dia. holes

12 - 1 in. Dia. holes 14.25 in. Bolt circle

17 in. Bolt circle

- * TYPICAL LOCATION FOR DISCHARGE NOZZLE
- ** FINAL HEAD HEIGHT WILL BE DETERMINED BASED ON INTERNAL ANALYSIS AND SPECIFICATION REVIEW

NOTES: ALL DIMENSIONS IN INCHES UNLESS OTHERWISE NOTED. DRAWING NOT TO SCALE.

NOT TO BE USED FOR CONSTRUCTION UNLESS CERTIFIED.

Customer: Ken	nedy .	Jenks		
Customer Refe	rence:			OUTLINE
Item Number: 0	12			DRAWING
Curve Number:	12JKI	H 1770		
Date: 06 Dec 20	016			
				DRAWING

Customer Technical Offer



Customer: Kennedy Jenks Reference:

Weir Floway Inc. SCORE 16.5.1.0

Item number012Size / Stages12JKH / 2Quote number480863Nominal pump speed1770 rpm

Totals

Grand Total \$ 68,741

Pump

Qty Description

2 Units - 12JKH - 2 stage Product lube - Barrel Pump

Pump selection criteria

Speed operation: Variable speed operation

Lubrication type

Lubrication type: Product lube

Bowl Assembly - 2 Stage

Bowl size: 12JKH bowl assembly - 2 stage

Bowl Materials: Cast iron (ASTM A48 cl 30-enamel lined)

Bowl connection type: Flanged

Bowl Bolting Material: 304SS (ASTM F593 Gr CW1), Floway material code - 106

Bowl bearing material: Bismuth tin bronze bowl bearings (UNS C89835)

Impeller Material: Bronze (ASTM B584 C90300) Collet Material: Steel (ASTM A108-90a Gr 1215)

Bowl Shaft Size: 1.6875" (Standard)

Bowl Shaft Material: 416SS (ASTM A582-88a Type 416)

Suction type: Suction bell

Suction type bearing: Bismuth tin bronze (UNS C89835)

Suction Strainer: Clip on basket strainer 12JK

Suction Strainer Material

Strainer material - Galvanized steel

Bowl assembly type: Fully assembled Column assembly - 1 x 8 in. - Threaded

Column

Column Size: Column 8" - (0- 20' and 0- 10' and 0- 5' and 1 - 3.85' Top)

Column pipe material: ASTM A53 Gr. B rolled and welded steel Column pipe schedule: Schedule 30 .277" wall thickness

Column Connection Type: Threaded

Lineshaft

Lineshaft Size: 1"

Lineshaft Material: 416SS (ASTM A582-88a Type 416)

Lineshaft Coupling Material: 416SS (ASTM A582-88a Type 416)

Line shaft bearing material: Styrene Butadiene Rubber(SBR) (Qty 0 per pump)

Discharge head assembly - 10x16.5x18 "VF"

Discharge head material: Steel (A36 plt, A105 flg, A53-Gr B pipe)

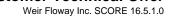
Discharge Head Size: 10x16.5x18 "VF"

Discharge size: 10"

Discharge Connection Type/Rating: 150# flange (Stl. std.)

Shaft sealing arrangement: Mechanical seal

Mechanical seal construction: Single unbalanced mechanical seal Mechanical seal type: John Crane type 5611 mechanical seal





Customer: Kennedy Jenks **Reference**:

Pump

Qty Description

Seal flush piping plan-Primary: Plan 13 Seal flush piping

Seal flush piping material - primary seal: 316SS tubing-Primary SFP

Top Line Shaft Straightness: Floway Standard

Stuffing box / Seal housing bearing material: Bismuth tin bronze seal housing bearing (UNS C89835)

Head shaft couplings: Type CPAT flanged adjustable spacer coupling

Coupling guard material / construction: Aluminum

Protective coatings

Protective coating - Discharge head: Carboquard 891 epoxy coating - Disch. head - interior and exterior

Protective coating - Column: Carboguard 891 epoxy coating - Column - interior and exterior Protective coating - Bowl assembly: Carboguard 891 epoxy coating - Bowls, exterior only

Protective coating - Barrel: Carboguard 891 epoxy coating - Barrel - interior only (exterior Carboline 635 primer)

Protective coating - Soleplate: Carboguard 891 epoxy coating - Soleplate top side only

Miscellaneous coating options

NSF certified

Assembly type - Unit

Assembly type - Unit: Factory assembled (bowl, head, and column only) shipped assembled

Start-up/Overage

Start-up options

Start up by Distributor/Manufacturer's Rep.

Packaging and Shipping

Packaging options

Domestic packaging

Testing

Qty Description

2 Testing and Inspection options

Performance / NPSH testing

Factory performance test acceptance criteria for rated condition per: ANSI/HI 14.6 grade 1U (Floway standard)

Performance test options

Bowl assembly performance test - 2 units

Performance test witnessing

Non-witnessed

Hydro testing

Hydrotest - Discharge Head options: Non witnessed hydrotest - discharge head - 2 units Hydrotest - Suction barrel options: Non witnessed hydrotest - suction barrel - 2 units

Inspection and Analysis

Analysis

Seismic analysis of anchorage

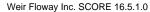
Structural natural frequency analysis (head/motor only), stamped by Floway P.E. - 1 units

Sole Plate

Qty Description

2 Discharge head assembly - 10x16.5x18 "VF"

Soleplate type: Fabricated steel Soleplate size: 36"x36"x1.25"





Customer: Kennedy Jenks **Reference**:

Anchor Bolt

Qty Description

2 Discharge head assembly - 10x16.5x18 "VF"

Soleplate anchor bolts with nuts: No soleplate anchor bolts

Barrel

Qty Description

2 Suction barrel 18 in. x 7.94 ft.

Suction barrel: Standard pressure suction barrel
Barrel diameter: 18" diameter suction barrel x 7.94 ft.
Barrel material: Steel barrel - ASTM A53 pipe A240 plate
Barrel suction nozzle: 12" suction nozzle and flange on barrel

Barrel suction flange rating: 150# suction flange

Driver

Qty Description

2 Driver

Electric motor driver

Motor size selection: US 50HP 460v/3ph/60hz 1800 RPM WPI

Motor efficiency type: Premium efficient

Motor shaft

Motor shaft type: Motor vertical solid shaft

Reference head shaft diameter: For reference:1" Top line shaft diameter

Motor thrust design High thrust

Motor bearing life options: 1 yr. min. / 5 yr. average

Motor enclosure: WPI Motor service factor: 1.15

Starting method: Across the line starting

Motor BD: Motor BD 12 in. Miscellaneous motor options

Thermostats
Inverter duty motor

Non-reverse device: No non-reverse device on motor

Motor testing options

Motor complete test - unwitnessed Conduit box size: Standard conduit box Elevation: Motor suitable for elevation <= 3300'

Ambient temperature: Motor suitable for ambient temperature <= 104 F (40 C)

UL labeled motor: Not UL labeled

Motor packaging options: Motor domestic packaging

Driver design: NEMA

Driver shipping options: Motor NOT to be shipped to Floway factory

	RMCC RECLAIMED/RAIN/RIVER WATER used FOR GOLF COURSE IRRIGATION													
	Jan	Feb	Mar	April	May	June	July	Aug	Sept	Oct	Nov	Dec	TOTAL	AC Feet
2004	0	0	0	0	0	0	0	32,271,664	24,124,682	12,042,621	0	0	68,438,967	210.0
2005	0	0	0	0	16,630,838	26,814,512	36,402,337	34,133,912	22,205,988	8,775,311	0	0	144,962,898	444.9
2006	0	0	0	0	6,766,725	33,466,274	34,890,191	29,922,670	25,027,177	4,124,965	251,454	0	134,449,456	412.6
2007	0	0	8,028,234	12,384,053	25,061,082	35,457,957	34,901,154	31,926,322	20,635,416	8,307,235	5,527,905	677,308	182,906,666	561.4
2008	1,659,642	3,416,483	7,124,928	18,287,541	29,461,199	34,964,198	33,603,413	31,014,257	24,379,703	9,898,221	558,332	0	194,367,917	596.5
2009	52,784	0	2,975,658	16,717,552	22,729,582	32,833,243	46,776,756	43,909,242	28,182,762	11,666,411	3,933,034	262,164	210,039,188	644.6
2010	597,420	531,726	519,342	1,149,164	12,408,766	37,970,917	46,140,605	40,058,609	27,082,893	11,123,674	3,537,359	175,506	181,295,981	556.4
2011	872,560	713,619	1,313,020	8,984,949	18,274,385	27,470,149	46,391,726	40,394,603	29,335,909	9,066,660	597,141	995,453	183,414,721	562.9
2012	878,154	2,778,006	1,196,596	7,361,960	32,770,815	45,143,654	47,147,006	42,805,041	28,569,713	12,850,329	492,614	15,155	221,993,888	681.3
2013	106,349	1,341,286	8,606,675	18,332,384	35,468,226	41,821,801	48,030,013	43,806,357	22,120,481	20,445,260	5,670,447	156,796	245,749,279	754.2
2014	3,376,895	770,891	5,676,877	15,768,648	32,126,458	43,082,072	45,349,608	44,684,082	26,637,494	12,584,964	757,116	148,932	230,815,105	708.4
2015	328,082	431,985	7,101,232	16,684,761	26,270,887	42,472,558	45,059,817	39,039,324	28,975,721	13,805,881	256,034	33,022	220,426,282	676.5
2016	13,823	0	0	0	0	0	0	0	0	0	0	0	13,823	0.0

From Master Wastewater Data Spreadsheet

Irrigation													
Season	July	Aug	Sep	Oct	Nov	Dec	Jan	Feb	March	April	May	June	Total
2004-2005	0	32,271,664	24,124,682	12,042,621	0	0	0	0	0	0	16,630,838	26,814,512	111,884,317
2005-2006	36,402,337	34,133,912	22,205,988	8,775,311	0	0	0	0	0	0	6,766,725	33,466,274	141,750,547
2006-2007	34,890,191	29,922,670	25,027,177	4,124,965	251,454	0	0	0	8,028,234	12,384,053	25,061,082	35,457,957	175,147,783
2007-2008	34,901,154	31,926,322	20,635,416	8,307,235	5,527,905	677,308	1,659,642	3,416,483	7,124,928	18,287,541	29,461,199	34,964,198	196,889,331
2008-2009	33,603,413	31,014,257	24,379,703	9,898,221	558,332	0	52,784	0	2,975,658	16,717,552	22,729,582	32,833,243	174,762,745
2009-2010	46,776,756	43,909,242	28,182,762	11,666,411	3,933,034	262,164	597,420	531,726	519,342	1,149,164	12,408,766	37,970,917	187,907,704
2010-2011	46,140,605	40,058,609	27,082,893	11,123,674	3,537,359	175,506	872,560	713,619	1,313,020	8,984,949	18,274,385	27,470,149	185,747,328
2011-2012	46,391,726	40,394,603	29,335,909	9,066,660	597,141	995,453	878,154	2,778,006	1,196,596	7,361,960	32,770,815	45,143,654	216,910,677
2012-2013	47,147,006	42,805,041	28,569,713	12,850,329	492,614	15,155	106,349	1,341,286	8,606,675	18,332,384	35,468,226	41,821,801	237,556,579
2013-2014	48,030,013	43,806,357	22,120,481	20,445,260	5,670,447	156,796	3,376,895	770,891	5,676,877	15,768,648	32,126,458	43,082,072	241,031,195
2014-2015	45,349,608	44,684,082	26,637,494	12,584,964	757,116	148,932	328,082	431,985	7,101,232	16,684,761	26,270,887	42,472,558	223,451,701
2015-2016	45,059,817	39,039,324	28,975,721	13,805,881	256,034	33,022	13,823						
Average	42,829,029	38,756,051	26,094,727	11,387,360	2,158,144	246,434	788,571	1,109,333	4,726,951	12,852,335	26,063,489	37,912,950	

AF/Month

Irrigation													
Season	July	Aug	Sep	Oct	Nov	Dec	Jan	Feb	March	April	May	June	Total
2004-2005	0	99	74	37	0	0	0	0	0	0	51	82	343
2005-2006	112	105	68	27	0	0	0	0	0	0	21	103	435
2006-2007	107	92	77	13	1	0	0	0	25	38	77	109	538
2007-2008	107	98	63	25	17	2	5	10	22	56	90	107	604
2008-2009	103	95	75	30	2	0	0	0	9	51	70	101	536
2009-2010	144	135	86	36	12	1	2	2	2	4	38	117	577
2010-2011	142	123	83	34	11	1	3	2	4	28	56	84	570
2011-2012	142	124	90	28	2	3	3	9	4	23	101	139	666
2012-2013	145	131	88	39	2	0	0	4	26	56	109	128	729
2013-2014	147	134	68	63	17	0	10	2	17	48	99	132	740
2014-2015	139	137	82	39	2	0	1	1	22	51	81	130	686
2015-2016	138	120	89	42	0.786	0.101	0.042						
Average	131	119	80	35	7	1	2	3	15	39	80	116	627

Irrigation § eason	July	Aug	Sep	Oct	Nov	Dec	Jan	Feb	March	April	May	June	Total
2004-2005	0	99	74	37	0	0	0	0	0	0	51	82	343
2005-2006	112	105	68	27	0	0	0	0	0	0	21	103	435
2006-2007	107	92	77	13	1	0	0	0	25	38	77	109	538
2007-2008	107	98	63	25	17	2	5	10	22	56	90	107	604
2008-2009	103	95	75	30	2	0	0	0	9	51	70	101	536
2009-2010	144	135	86	36	12	1	2	2	2	4	38	117	577
2010-2011	142	123	83	34	11	1	3	2	4	28	56	84	570
2011-2012	142	124	90	28	2	3	3	9	4	23	101	139	666
2012-2013	145	131	88	39	2	0	0	4	26	56	109	128	729
2013-2014	147	134	68	63	17	0	10	2	17	48	99	132	740
2014-2015	139	137	82	39	2	0	1	1	22	51	81	130	686
2015-2016	138	120	89	42	0.786	0.101	0.042						
Average of last 10 yr	130	120	80	35	5	0	0	5	15	40	80	115	625
Maximum	145	135	90	65	15	5	10	10	25	55	110	140	740
Minimum	105	90	65	15	0	0	0	0	0	5	40	85	535
AVERAGE 2006-2008 2008-2010 2010-2012	107 123 142	95 115 123	70 81 87	19 33 31	9 7 6	1 0 2	3 1 3	5 1 5	23 5 4	47 27 25	84 54 78	108 109 111	571 557 618
2012-2014	146	133	78	51	9	0	5	3	22	52	104	130	734
2014-2016	139	128	85	40	2	0	1	1	22	51	81	130	686
AVERAGE	130	120	80	35	5	0	0	5	15	40	80	120	635
Maximum	146	133	87	51	9	2	5	5	23	52	104	130	734
Minimum	107	95	70	19	2	0	1	1	4	25	54	108	557
SUM													
2006-2008	214	190	140	38	18	2	5	10	47	94	167	216	1,142
2008-2010	247	230	161	66	14	1	2	2	11	55	108	217	1,113
2010-2012	284	247	173	62	13	4	5	11	8	50	157	223	1,236
2012-2014	292	266	156	102	19	1	11	6	44	105	207	261	1,469
2014-2016	277	257	171	81	3	1	1	1	22	51	81	130	686
AVERAGE	265	240	160	70	15	0	5	5	25	70	145	210	1,130
Maximum	292	266	173	102	19	4	11	11	47	105	207	261	1,469
Minimum	214	190	140	38	3	1	1	1	8	50	81	130	686

^{*}Peak month is July according to the Averages outlined in blue

Irrigation § eason	July	Aug	Sep	Oct	Nov	Dec	Jan	Feb	March	April	May	June	Total
PERCENTAGE	_												
2006-2008	18.76	16.62	12.27	3.34	1.55	0.18	0.45	0.92	4.07	8.24	14.66	18.93	
2008-2010	22.16	20.66	14.49	5.95	1.24	0.07	0.18	0.15	0.96	4.93	9.69	19.52	
2010-2012	22.98	19.98	14.01	5.01	1.03	0.29	0.43	0.87	0.62	4.06	12.68	18.03	
2012-2014	19.89	18.10	10.59	6.96	1.29	0.04	0.73	0.44	2.98	7.13	14.12	17.74	
2014-2016	40.46	37.47	24.89	11.81	0.45	0.08	0.15	0.19	3.18	7.47	11.76	19.01	
AVERAGE	24.85	22.57	15.25	6.61	1.11	0.13	0.39	0.51	2.36	6.36	12.58	18.65	
Maximum													
Minimum													

NUMBER OF CONNECTIONS TIMELINE

		Number of Co	onnections			Developme	nt Timeline				
	Developments	Residential	Commercial	2016	2018	2020	2025	2030	2035	2040	2045
	Existing (Current)	2,502									
Phase 1	Infill	238			238	0.05					
	Retreats North and East	62			62	0.01302					
	Retreats West	22			22	0.00462					
	Murieta Gardens	78	227		78	0.06416					
	Phase 1 Alon	e 400				0.1318					
	Total (Phase 1	L) 2,902				0.4718					
	% Increase from Currer	nt 16%									
Phase 2											
	Village A	167				117	25	12	13		
	Village B	167				17	50	50	50		
	Village C	130				13	52	52	13		
	Village D	42				0	11	11	21		
	Village E	43				0	0	9	34		
	Village F	95				0	2	36	57		
	Village G	53				0	0	5	48		
	Village H	122				0	12	31	79		
	Riverview	140				140					
	Lakeview	99				99					
	Apartments	170				119	26	12	14		
	Residences of Murieta Hills	198				198					
	Industrial/Commercial/Residential	160	_			24	48	48	40		
	Phase 2 Alon	e 1,586		2,502	400	727	225	265	369	0	0
	Total (Phase 2	2) 4,488		2,502	2,902	3,629	3,854	4,119	4,488	4,488	4,488
	% Increase from Currer	nt 79%									
			ADWF (MGD)	0.3400	0.4718	0.624346	0.671649	0.727233	0.80482	0.80482	0.80482

Source

See AD Demand and Sources; 0.5 MGD allocation for infill

Draft Sewer Study May 6, 2016 & Preliminary Sewer Study May 31, 2016

Final Sewer Study, May 3, 2016

Draft Sewer Study, May 15, 2016; Commercial connections based on 0.04774 MGD and 210 gpd/connection (Table 2)

Prelim Sewer Study, March 31, 2016, Section 5. Development Timeline per page 4 of RMCSD Water Supply Assessment TM Prelim Sewer Study, March 31, 2016, Section 5. Development Timeline per page 4 of RMCSD Water Supply Assessment TM Prelim Sewer Study, March 31, 2016, Section 5. Development Timeline per page 4 of RMCSD Water Supply Assessment TM Prelim Sewer Study, March 31, 2016, Section 5. Development Timeline per page 4 of RMCSD Water Supply Assessment TM Prelim Sewer Study, March 31, 2016, Section 5. Development Timeline per page 4 of RMCSD Water Supply Assessment TM Prelim Sewer Study, March 31, 2016, Section 5. Development Timeline per page 4 of RMCSD Water Supply Assessment TM Prelim Sewer Study, March 31, 2016, Section 5. Development Timeline per page 4 of RMCSD Water Supply Assessment TM Prelim Sewer Study, March 31, 2016, Section 5. Development Timeline per page 4 of RMCSD Water Supply Assessment TM Prelim Sewer Study, March 31, 2016, Section 5. Development Timeline per page 4 of RMCSD Water Supply Assessment TM Title XVI Recycled Water Feasibility Study, June 2014. Page 2-5, Table 2-1

Title XVI Recycled Water Feasibility Study, June 2014. Page 2-5, Table 2-2

Title XVI Recycled Water Feasibility Study, June 2014. Page 2-5, Table 2-3

Preliminary Sewer Study, March 31, 2016, Section 5

Title XVI Recycled Water Feasibility Study, June 2014. Page 2-5, Table 2-3

RECYCLED WATER DEMAND TIMELINE

				I	Developme	nt Timeline					
	RW Annual Dema	and (AFY)	2016	2018	2020	2025	2030	2035	2040	2045	
North & South Golf Courses (Current)	550		550	550	550	550	550	550	550	550	
Infill	0			0	0	0	0	0	0	0	
North Main Gate Entrance (Phase 1)	2.8				2.8	2.8	2.8	2.8	2.8	2.8	
District Office	5.4				5.4	5.4	5.4	5.4	5.4	5.4	
Retreats (Phase 1)	15.1			15.1	15.1	15.1	15.1	15.1	15.1	15.1	
Murieta Gardens (Phase 1)	30.5			30.5	30.5	30.5	30.5	30.5	30.5	30.5	
Stonehouse Park (Phase 1)	36.2				36.2	36.2	36.2	36.2	36.2	36.2	
Escuela Park (Phase 1)	12.07				12.07	12.07	12.07	12.07	12.07	12.07	_
Phase 1 Alone	e 102.07		550.00	595.60	652.07	652.07	652.07	652.07	652.07	652.07	
Total (Phase 1	652.07										
Village A (Phase 2)	56.5				39.56107	8.477373	3.956107	4.521266			56.515
Village B (Phase 2)	64.6				6.457333	19.372	19.372	19.372			64.573
Village C (Phase 2)	49.6				4.963636	19.85455	19.85455	4.963636			49.636
Village D (Phase 2)	0.00				0	0	0	0			
Village E (Phase2)	0.00				0	0	0	0			
Village F (Phase 2)	0.00				0	0	0	0			
Village G (Phase 2)	0.00				0	0	0	0			
Village H (Phase 2)	0.00				0	0	0	0			
Riverview	0.00				0	0	0	0	0		
Lakeview	0.00				0	0	0	0	0		
Apartments (Phase 2)	23.8				16.66	3.57	1.666	1.904			2
Residences of Murieta Hills (Phase 2)	73.8				73.8	0	0.00	0.00	0	0	7
Industrial/Commercial/Residential (Phase 2)	50.9				7.64	15.27	15.27	12.73			5
Phase 2 Alone	e 319.2	•	0.0	0	149.077	66.54392	60.12	43.49	0	0	
Total (Phase 2	971.29	SUM	550	595.60	801.14	718.61	712.19	695.55	652.07	652.07	
		COMPOUNDING SUM	550	595.60	801.14	867.69	927.81	971.29	971.29	971.29	

WASTEWATER PRODUCTION TIMELINE

				D	evelopmen	t Timeline				
	Waswater Produc	tion (AFY)	2016	2018	2020	2025	2030	2035	2040	2045
North & South Golf Courses (Current)	380.87		380.87	380.87	380.87	380.87	380.87	380.87	380.87	380.87
Infill	56.0			56.0	56.0	56.0	56.0	56.0	56.0	56.0
North Main Gate Entrance (Phase 1)	0.0				0.0	0.0	0.0	0.0	0.0	0.0
District Office	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Retreats (Phase 1)	19.8			19.8	19.8	19.8	19.8	19.8	19.8	19.8
Murieta Gardens (Phase 1)	71.9			71.9	71.9	71.9	71.9	71.9	71.9	71.9
Stonehouse Park (Phase 1)	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Escuela Park (Phase 1)	0.0				0.0	0.0	0.0	0.0	0.0	0.0
Phase 1 Alo	ne 147.6		380.87	528.51	528.51	528.51	528.51	528.51	528.51	528.51
Total (Phase	1) 528.5									
Village A (Phase 2)	39.3				27.50	5.89	2.75	3.14		
Village B (Phase 2)	39.3				3.93	11.79	11.79	11.79		
Village C (Phase 2)	30.6				3.06	12.23	12.23	3.06		
Village D (Phase 2)	9.9				0.00	2.47	2.47	4.94		
Village E (Phase 2)	10.1				0.00	0.00	2.02	8.09		
Village F (Phase 2)	22.3				0.00	0.45	8.49	13.41		
Village G (Phase 2)	12.5				0.00	0.00	1.25	11.22		
Village H (Phase 2)	28.7				0.00	2.87	7.18	18.66		
Riverview	32.9				32.93	0.00	0.00	0.00	0.00	
Lakeview	21.4				21.40	0.00	0.00	0.00	0.00	
Apartments (Phase 2)	23.3				16.30	3.49	1.63	1.86		
Residences of Murieta Hills (Phase 2)	46.6				46.58	0.00	0.00	0.00	0.00	0.00
Industrial/Commercial/Residential (Phase 2) 37.6				5.65	11.29	11.29	9.41		
Phase 2 Alo	ne 354.5	-	0.0	0.00	157.35	50.48	61.10	85.58	0.00	0.00
Total (Phase	2) 883.0	SUM	380.87	528.51	685.86	578.99	589.61	614.09	528.51	528.51
		COMPOUNDING SUM	380.87	528.51	685.86	736.34	797.44	883.02	883.02	883.02

		2020	2025	2030	2035	2040	2045
	Village A	70%	15%	7%	8%		
	Village B	10%	30%	30%	30%		
	Village C	10%	40%	40%	10%		
	Village D	0%	25%	25%	50%		
	Village E	0%	0%	20%	80%		
	Village F	0%	2%	38%	60%		
	Village G	0%	0%	10%	90%		
	Village H	0%	10%	25%	65%		
	Riverview	100%	0%	0%	0%	0%	0%
	Lakeview	100%	0%	0%	0%	0%	0%
56.51582	Apartments	70%	15%	7%	8%		
64.57333	Residences of Murieta Hills	100%	0%	0%	0%	0%	0%
49.63636	Industrial/Commercial/Residential	15%	30%	30%	25%		
0							

0 Existing Wastewater Flow (ADWF MGD)

0 0 23.8 73.8 50.9 0.3400

Table 1. Projected Average Annual Recyled Water Demands and Scenarios

Down and Downley was to and Down Asset	Projected RW Demand		Scenarios (AFY)	
Proposed Developments and Reuse Area	(AFY)	1- WDR and RW Standards	2- Public Area Focus, Limited to Most Cost Effective	3-Scenario 2 Plus Riverview and Lakeview
North and Sourth Golf Courses (Current)	550	550	550	550
North Main Gate Entrance (Phase 1)	2.8	2.8	2.8	2.8
District Office (Phase 1)	5.4	5.4	5.4	5.4
Stonehouse Park (Phase 1)	36.2	36.2	36.2	36.2
Escuela Park (Phase 1)	12.1	12.1	12.1	12.1
Commercial Loop (TBD)			10	10
Retreats (Phase 1)	15.1	15.1	15.1	15.1
Murieta Gardens (Phase 1)	30.5	30.5	30.5	30.5
Village A (Phase 2)	56.5	56.5		
Village B (Phase 2)	64.6	64.6		
Village C (Phase 2)	49.6	49.6	49.6	49.6
Apartments (Phase 2)	23.8	23.8	23.8	23.8
Residences of Murieta Hills (Phase 2)	73.8	73.8	73.8	73.8
Industrial/Commercial/Residential (Phase 2)	50.9	50.9	50.9	50.9
Village D				
Village E				
Village F				
Village G				
Village H				
Riverview	22.4			22.4
Lakeview	15.8			15.8
Sum of Prop	osed Reuse Area Demands	971	860	898
Projected I	Recycled Water Production	883	883	883
Differer	nce (Excess Recycled Water	-88	23	-15

Notes:

Developments with phase descriptions (i.e., Phase 1 and 2) reflect proposed reuse areas described in the District's Waste Discharge Requirements and Recycled Water Standards

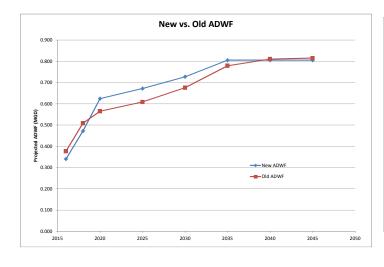
ADWE and Developments Comparison

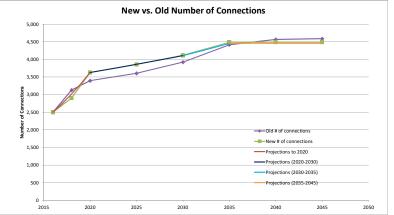
OLD - From Park and Common	Area Irrigation	Damande enroadchoot

Conditions	Source	Numbe	r of Connections Wastewa	iter Flow, (ADWF MGD) Recycled Wa	iter Demand (AFY)				
Existing	District Engineer RFP		2500	0.37653	0				
Phase 1 - Connected Prior to 2020	Current Activities								
Murieta Gardens		305							
The Retreats		84							
Infill - 0.05 MGD ADWF		238							
Phase 2 - Per WSA	per Water Supply Assessment, Ta	ible 2-1							
	2016	2018	2020	2025	2030	2035	2040	2045	
Village A			117	25	12	13			
Village B			17	50	50	50			
Village C			13	52	52	13			
Village D			0	10	11	21			
Village E			0	0	9	34			
Village F			0	2	36	57			
Village G			0	0	5	48			
Village H			0	13	32	85			
Industrial/Commercial/Residential			4	8	7	6			
Residences of MH			0	0	20	79	79	20	
Riverview			0	14	42	42	42		
Lakeview			0	9	30	30	30		
Apartments			119	25	12	14			
		627	270	208	318	492	151	20	
	2500	3,127	3397	3605	3923	4415	4566	4586	
existing ADWF + Proposed Connection ADWF									
compounded each 5 years)	0.37653	0.50825	0.56495	0.60863	0.67541	0.77873	0.81044	0.81464	

IVEVV								
connections	2,502	2,902	3,629	3,854	4,119	4,488	4,488	4,488
ADWF	0.3400		0.6243	0.6716	0.7272	0.8048	0.8048	0.8048
Connections DIFFERENCE	-2		-231	-249	-195	-73	78	98
ADWF DIFFERENCE	0.03653		-0.05939	-0.06302	-0.05182	-0.02609	0.00562	0.00982

Projections to 2020		Projection	ns from 2020 to 2030	Projection	ns from 2030 to 2035	Projection	ns from 2035 to 2045
assumption	9.75%	-	assumption 1.2	5% a	ssupmtior 1.65%	ass	umption 0.00%
year	connections	year	connecti	ons year	connections	year	connections
2016	2,502	2020	3,629	00 2030	4,107.00	2035	4,458.00
2017	2,745.95	2021	3,674	36 2031	4,174.77	2036	4,458.00
2018	3,013.67	2022	3,720	29 2032	4,243.65	2037	4,458.00
2019	3,307.51	2023	3,766	80 2033	4,313.67	2038	4,458.00
2020	3,629.99	2024	3,813	88 2034	4,384.84	2039	4,458.00
		2025	3,861	55 2035	4,457.19	2040	4,458.00
		2026	3,909	82		2041	4,458.00
		2027	3,958.	70		2042	4,458.00
		2028	4,008	18		2043	4,458.00
		2029	4,058	28		2044	4,458.00
		2030	4,109	01		2045	4,458.00
di	fferemce 1,128		difference 4	80 d	ifferemce 350		





9-hr	8-hr		Recycled Water Deman		
33	37	9,428	North Main Gate Entrance	North	Phase 1
17	19	9,120	sin District Office	zation Basi	Equalia
117	132	63,360	Retreats	North	
188	211	101,280	Murieta Gardens	North	
223	251	120,480	Stonehouse Park	North	
223	251	120,480	Escuela Park	North	
1,871	2,104		North Golf Course	North	
1,703	1,915		South Golf Course	South	
4,375	4,920	Phase 1 Demand			
5,552	6,246	Capacity			
					Phase 2
396	446	214,080	Village A	North	
306	344	165,120	Village C	North	
348	392	188,160	Village B	North	
148	167	80,160	Apartments	North	
460	518	248,640	Residences of Murieta Hills	North	
317	357	171,360	Industrial/Commercial/Residential	North	
1,975	2,224	Phase 2 Subtotal			
6,350	7,144	Phase 1 and 2 Total			
5,552	6,246	Capacity			
798	898	Difference (Supplemental Potable Water), gpm			
430,718	431,258	Difference (Supplemental Potable Water), gallons			

RW Product	tion Sources		
WWRP	2,082	2,082	
Potable Water Supplementation	896	796	
Subtotal	2,978	2,878	
Reduced GC Demand (assumed)	1000	1000	
	943	496	
	452,458	268,018	Phase
	3,167	2,471	Phase
	1,519,978	1,186,238	

	RW Annual Demand (AFY)	RW Average Day Demand (AF/day)	RW Average Day Demand (MGD)	Waswater Production (AFY)	Area (AC)
1 North & South Golf Courses (Current)	550	2.782	0.9065	381	
2 Infill	0			56	
3 North Main Gate Entrance (Phase 1)	2.8	0.014	0.0046	0	
4 District Office	5.4	0.027	0.0089	0	
5 Retreats (Phase 1)	15.1	0.076	0.0249	19.8	
6 Murieta Gardens (Phase 1)	30.5	0.154	0.0503	71.9	
7 Stonehouse Park (Phase 1)	36.2	0.183	0.0597	0	
8 Escuela Park (Phase 1)	12.07			0	
Phase 1 Alone	102			148	
Total (Phase 1)	652		1.05	529	
% Increase from Current	19		16	39	
9 Village A (Phase 2)	56.5	0.286	0.0931	39.3	94.5
10 Village B (Phase 2)	64.6	0.327	0.1064	39.3	81.7
11 Village C (Phase 2)	49.6	0.251	0.0818	30.6	63.3
12 Village D (Phase 2)	0		0	9.9	107.6
13 Village E (Phase 2)	0		0	10.1	
14 Village F (Phase 2)	0		0	22.3	77.1
15 Village G(Phase 2)	0		0	12.5	182.3
16 Village H (Phase 2)	0		0	28.7	
17 Riverview	0	0.000	0	32.9	57.4
18 Lakeview	0	0.000	0	21.4	41.6
19 Apartments (Phase 2)	23.8	0.120	0.0392	23.3	17.8
20 Residences of Murieta Hills (Phase 2)	73.8	0.373	0.1216	46.6	168.7
21 Industrial/Commercial/Residential (Phase 2)	50.9	0.257	0.0839	37.6	39.5
Phase 2 Alone	319			355	
Total (Phase 2)	971		1.58	883	
% Increase from Current	77		74	132	

Balance of Average Day Demands and Sources

,	MGD
WWRP Capacity (Current)	2.3
WWRP Capacity (Phase 1)	3.0
Min. Supplemental Potable Water Requirements (AFY)	150
Current	169
Phase 1	124
Phase 2	88
WWRP Capacity Difference after Phase 1	1.95
WWRP Capacity Difference after Phase 2	1.42

Notes

- 1 Current golf course demands and ADWF of 0.34 MGD as described in Retreats West Capacity Certification Letter
- 2 To be determined; 0.05 MGD ADWF allocation
- 3 RW Demand obtained from Table 5 of the June 2016 Recycled Water Modeling Study Report
- 4 RW Demand obtained from Table 5 of the June 2016 Recycled Water Modeling Study Report
- 5 Values obtained from latest K/J comments on Retreatas North and East Sewer Study (July 19, 2016) and Retreats West Capacity Certification Letter
- 6 Values obtained from May 15, 2016 Murieta Gardens I & II Sewer Study currently under review
- 7 Value obtained from Table 5 of the June 2016 Recycled Water Modeling Study Report
- 8 RW obtained from Table 5; Escuela Park does not include any homes and occupies the entire site; wastewater production = 0
- 9 Recycled Water Demand derived from Table 5-1 of the Title XVI Recycled Water Feasibility Study and ratio of current (167) to previous (177) future number of residential homes. Wastewaster production based on 210 gpd/connection
- 10 Recycled Water Demand derived from Table 5-1 of the Title XVI Recycled Water Feasibility Study and ratio of current (167) to previous (120) future number of residential homes. Wastewaster production based on 210 gpd/connection
- 11 Recycled Water Demand derived from Table 5-1 of the Title XVI Recycled Water Feasibility Study and ratio of current (130) to previous (110) future number of residential homes. Wastewaster production based on 210 gpd/connection
- **12** N/A
- **13** N/A
- **14** N/A
- **15** N/A
- **16** N/A
- 17 RW Demand obtained from Table 5 of the June 2016 Recycled Water Modeling Study Report. Wastewater production value obtained from Table 5
- 18 RW Demand obtained from Table 5 of the June 2016 Recycled Water Modeling Study Report. Wastewater production value obtained from Table 5
- 19 Values obtained from Table 5 of the June 2016 Recycled Water Modeling Study Report
- 20 Values obtained from Table 5 of the June 2016 Recycled Water Modeling Study Report
- 21 Values obtained from Table 5 of the June 2016 Recycled Water Modeling Study Report

Maximum	Month/Da	v Demands

	MGD	AF/Month	_	
North&South Golf Courses (Current)	1.852	172.9	_	
North Main Gate Entrance (Phase 1)	0.009	0.9		
District Office	0.018	1.7		
Retreats (Phase 1)	0.051	4.7		
Murieta Gardens (Phase 1)	0.103	9.6		
Stonehouse Park (Phase 1)	0.122	11.4		
Escuela Park (Phase 1)	0.041	3.8		
Total (Phase 1)	2.20	205.0	6.73848	16 Percent increase over existing GC demand
Village A (Phase 2)	0.190	17.8	204.241	
Village B (Phase 2)	0.217	20.3		
Village C (Phase 2)	0.167	15.6		
Apartments (Phase 2)	0.080	7.5		
Residences of Murieta Hills (Phase 2)	0.248	23.2		
Industrial/Commercial/Residential (Phase 2)	0.171	16.0		
Total (Phase 2)	3.27	305	10.03737	43 Percent increase over existing GC demand
			304.2292	
Maximum Month/Day Sources				
	MGD	•		
WWRP Capacity (Current)	2.3			
\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	2.0			

0

	MGD			
WWRP Capacity (Current)	2.3			
WWRP Capacity (Phase 1)	3.0			
Supplemental Potable Water Requirements (MGD)				
Current	0.000			
Phase 1	0.00			
Phase 2	0.27			

No supplemental water required Production (2.3 MGD) > Demand (2.2 MGD); no supplemental water required Production (3.0 MGD) < Demand (3.27 MGD); supplemental water required -> 230,000 gallons per day or 21.5 AF/month

430000 1.319711 30.30966

TION DEMANDS AND SOURCES				
Sources of Recycled Water				
		Capacity	y (MGD)	Notes
Location	Volume (MG)	8-hr IRR	9-hr IRR	
WWRP - Equalization Basin / North RW Pump Station (Current	1.8	1,596	1,596	
WWRP - Equalization Basin / North RW Pump Station (Phase 1	1.8	2,082	2,082	
Lookout Hill Tank (Phase 1)	0.1	104	93	Standards: 50% avaliable for production/meeting IRR demand
Bass Lake Tank (Phase 2)	0.5	521	463	Standards: 50% avaliable for production/meeting IRR demand
Supplemental Potable Water Supply (Phase 1)		898	798	
Golf Course Ponds (reduced rate of RW supply during IRR)				
Bass Lake (Phase 1)	12.1	2,104	1,870	6.2 acres, 6 ft average depth. Capacity based on 6 in draw down (happens to balance with feed rates)
Lakes 10, 11, 16 and 17 (Phase 2)	15.6	1,878	1,669	8.3 acres total, various depths. Capacity based on 4 inch draw down (close to balancing with feed rates)
Total (Curre	nt)	1,596	1,596	
Total (Future - Phase	1)	5,189	4,843	
Total (Future - Phase	2)	7,588	6,975	
Peak Demands of Recycled Water		Deman	d (gpm)	
	Volume (gpd)	8-hr IRR	9-hr IRR	
North Golf Course (Current)	1,010,138	2,104	1,871	
South Golf Course (Current)	841,782	1,754	1,559	
North Main Gate Entrance (Phase 1)	9,428	20	17	
District Office	18,182	38	34	
Retreats (Phase 1)	50,844	106	94	
Murieta Gardens (Phase 1)	102,697	214	190	
Stonehouse Park (Phase 1)	121,890	254	226	
Escuela Park (Phase 1)	40,630	85	75	
	Total (Phase 1)	4,574	4,066	
Village A (Phase 2)	190,296	396	352	
Village B (Phase 2)	217,427	453	403	
Village C (Phase 2)	167,132	348	310	
Apartments (Phase 2)	80,138	167	148	
Residences of Murieta Hills (Phase 2)	248,494	518	460	
Industrial/Commercial/Residential (Phase 2)	171,387	357	317	
Т	otal (Phase 1 and 2)	6,813	6,056	

Kevin Kennedy

Paul Siebensohn <psiebensohn@ranchomurietacsd.com> Tuesday, July 19, 2016 3:46 PM Kevin Kennedy Pond volumes

From: Sent: To: Subject:

fyi...I put this together a while ago for all of our bodies of water.

Name	2	Surface Area (acres)
1)	Calero	110 -114 acres, 2622 acre-feet volume
2)	Chesbro	62- 64 acres, 1130.7 acre-feet
3)	Clementia	71-76 acres, 907.1 acre-feet
4)	Laguna Joaquin	21.53 – 24.07 acres, 122 acre-feet
5)	Basin 5	1.3 acres at 16.5 foot average depth
6)	Guadalupe	1.3 acres
7)	Bass Lake	6.2 acres, 6 foot average depth
8)	Hole 10 North Pond	1.0 acres, 4 foot average depth
9)	6B Basin	0.2 acres, 4.6 foot average depth
10)	South Hole 10 Pond	1.4 acres, 5 feet average depth
11)	South Hole 11 Pond	6.3, 5.5 foot average depth
12)	South Hole 6 North Pond	0.4 and 0.28 acres
13)	South Hole 16 Pond	0.34 acres, >10 foot depth
14)	South Hole 17 Pond	0.27 acres, >10 foot depth
15)	North Hole 2 Pond	0.34 acres, 3.4 foot average depth

area, acres ave depth, vol, AF vol, MG						
6.2	6	37.2	12.1			
1.4	5	7	2.3			
6.3	5.5	34.65	11.3			
0.34	10	3.4	1.1			
0.27	10	2.7	0.9			
8.3			15.6			

Paul Sigbgnsohn Director of Field Operations Rancho Murieta CSD ph.(916)354-3700

Obtained from Recycled Water Feasiblity Study (HDR, June 2009) Figure 3

_	Monthly AF Demand	% of Total Demand	AF/Mnth	# days/Mnth	MGD
15-Jan	0	0.0	0.0	31	0
14-Feb	0	0.0	0.0	28	0
15-Mar	2.5	1.8	15.3	31	0.160641
15-Apr	5	3.5	30.6	30	0.331991
15-May	15	10.6	91.7	31	0.963845
15-Jun	23	16.3	140.6	30	1.527159
15-Jul	27.5	19.5	168.1	31	1.767049
14-Aug	28	19.9	171.2	31	1.799177
14-Sep	20	14.2	122.3	30	1.327964
14-Oct	12.5	8.9	76.4	31	0.803204
14-Nov	7.5	5.3	45.9	30	0.497987
14-Dec	0	0.0	0.0	31	0
	141	100	862	365	



